Green Valley Special Utility District Wastewater Treatment Capacity Summary Most Downstream Drainage Basin Location								
Design Flow	1 EDU/Acre Capacity (MGD)	3 EDU/Acre Capacity (MGD)						
Drainage Basin A	2.3	6.8						
Drainage Basin B	1.1	3.4						
Drainage Basin C	1.4	4.1						
Drainage Basin D	1.6	4.9						
Drainage Basin E	8.7	26.2						
Drainage Basin F	1.6	4.8						
Drainage Basin G	1.8	5.5						

The above table shows the wastewater capacity per 1 and 3 EDU/Acre development densities at the most downstream location in each drainage basin.

The exhibits in the attachments show theoretical wastewater treatment plants distributed along major trunk lines in each drainage basin. As the plants locate further downstream, there is a need for larger wastewater treatment plants. This shall be useful as a working guide to quickly estimate wastewater treatment capacity and costs at any pointy in the GVSUD wastewater collection system.

(see **Attachment 1,** Exhibit 5 - GVSUD Proposed Wastewater Treatment Capacity and Cost vs. Downstream Locations)

(see Attachment 2, Exhibit 4 – Wastewater Treatment Plant Capacity and Costs)

6.0 <u>Capital Improvement Projects (CIP)</u>

Presently, there are no GVSUD CIP projects to discuss. Each major trunk line along with the associated costs can be considered a preliminary list of CIP. As further communication with the development a community transpires, GVSUD shall develop list of CIP.

The adoption of the Harvest Hills Wastewater Treatment Plant could be considered an initial GVSUD Wastewater CIP.

7.0 <u>Discharge Options and Permits</u>

There are three basic types of effluent discharge permits. There is land application type, or zero discharge, where the effluent is discharged onto the surface of the land for evaporation or further filtration through the soil. This application is the easier permit to obtain from the TCEQ. However, this permit requires a great deal of land space that cannot be used for residential purpose.

The second type of permit is an effluent discharge permit. This type of permit allows wastewater effluent to be discharged into surface waters. This type of permit does not require as much land to support, but takes a great deal of time and is not a simple process to obtain a permit with TCEQ.

The third type of effluent discharge would be an opportunity for reuse or possibly sell the effluent. These options have revenue merit as well as benefit to society merit. There could be a great deal of costs associated with this type of discharge if there is not a reuse facility conveniently located nearby. The opportunity for wastewater effluent reuse would require additional study on a per project basis. As wastewater opportunities begin to surface, GVSUD should always explore wastewater effluent reuse opportunities from both customer service and potential additional revenue stream.

8.0 Estimated Costs

8.1 Wastewater Main Collection System Costs

For the purpose of determining long range feasibility and probable impact fees, a cost estimate was prepared for the wastewater collection system for Drainage Areas A through G. The estimates include costs associated with construction, basic engineering, easement acquisition, and survey. The cost estimates are shown in the below tables represent estimated cost for the proposed main wastewater collection system. The table identifies the costs of wastewater collection system per 1 and 3 EDU/Acre development densities.

Green Valley Special Utility District Summary Costs Proposed Main Wastewater Collection System Engineer's Opinion of Probable Costs										
Basin	Total C	costs 1 (EDU/acre)	Tota	I Costs 3 (EDU/acre)		Variance				
Α	\$	11,212,950.00	\$	13,229,734.00	\$	2,016,784.00				
В	\$	3,379,449.00	\$	3,848,841.00	\$	469,392.00				
С	\$	4,151,280.00	\$	4,773,440.00	\$	622,160.00				
D	\$	3,072,068.00	\$	4,188,876.00	\$	1,116,808.00				
E	\$	34,601,813.00	\$	43,682,177.00	\$	9,080,364.00				
F	\$	5,230,109.00	\$	6,739,925.00	\$	1,509,816.00				
G	\$	3,963,086.00	\$	4,673,334.00	\$	710,248.00				
Total	\$	65,610,755.00	\$	81,136,327.00 experience and qualifications	\$	15,525,572.00				

This cost estimate is based on River City Engineering's experience and qualifications, and represents River City Engineering's best judgment. This cost estimate was prepared for feasibility analysis purposes only. River City Engineering does not guarantee that the actual construction cost will not vary from this estimate. Unit prices were used from SAWS average unit price list revised October 2005. Units prices will not remain constraint and will vary due to market variations such as inflation.

(see **Attachment 2**, Exhibit 3 Wastewater Collection System Costs for unit prices)

8.2 Wastewater Treatment Plant Costs

For the purpose of determining long range feasibility and probable impact fees, a cost estimate was prepared for the wastewater treatment plant facilities. The wastewater treatment plant facilities were located at several locations along the downstream path of the main trunk line through each watershed or drainage basin. The estimates include the capacity of the plant if the plant was to be located at the location shown on These plant locations are not intended to represent actual the trunk line. recommended locations for wastewater treatment plants, but do represent what capacity and associated costs would exist periodically down the stream of the trunk line. The costs estimates includes construction, basic engineering, easement acquisition, and The cost estimates shown (Attachment 1 Exhibit 5 GVSUD proposed wastewater treatment capacity and costs vs. downstream locations) represent theoretical wastewater treatment plant locations, required capacity at the assumed location, and costs of proposed wastewater treatment facilities. This method allows GVSUD to estimate wastewater treatment capacity and costs anywhere along the collection system.

The following table represents wastewater treatment costs at the most downstream location within each drainage basin.

Green Valley Special Utility District Wastewater Treatment Costs Summary Most Downstream Drainage Basin Location								
Design Flow		1 EDU/Acre Costs (\$)	3 EDU/Acre Costs (\$)					
Drainage Basin A	\$	7,898,433	\$	20,310,255				
Drainage Basin B	\$	4,021,675	\$	12,065,025				
Drainage Basin C	\$	4,832,870	\$	14,498,610				
Drainage Basin D	\$	5,734,103	\$	17,202,308				
Drainage Basin E	\$	26,179,965	\$	39,269,948				
Drainage Basin F	\$	5,586,613	\$	16,759,838				
Drainage Basin G	\$	6,440,683	\$	16,561,755				

(see **Attachment 2**, Exhibit 4, Wastewater Treatment Plant Capacity and Costs for unit prices (Costs/GPD)

From the above table the required cost to provide wastewater treatment is substantially different between 1 EDU/Acre and 3 EDU/Acre development densities. The actual wastewater capacity and costs will be somewhere between these two EDU densities.

9.0 Proposed Financing Opportunities

GVSUD has several financing options when determining projects. The projects can be funded through equity, debt, or arrangements with the development community. For the debt arrangement, GVSUD could utilize the existing revenue streams from the water service to raise funds for wastewater. However, due to the political disagreement with water users who do not get wastewater services, this would probably not help GVSUD.

Through legislation, GVSUD could also designate portions of the GVSUD wastewater CCN service area as a taxing entity. This could also cause political disagreement with water service customers who do not get wastewater service.

9.1 Community Development Block Grant, Rural Development — U. S. Department of Agriculture

The U. S. Department of Agriculture provides loans and grants for water and sanitary sewer projects through its Rural Development Program. First time water and sanitary sewer service projects usually receive favorable consideration.

The Rural Development Program has a Colonias grant program for which the entire amount of the requested funds is allowed. To be eligible for this program, the community has to be a declared or listed "Colonia". Further study is required to determine if any areas within the GVSUD wastewater CCN service area qualifies.

The typical grant program for this agency requires a match from the applicant. The match amount may vary from between 25 and 40 percent of the amount of the grant. We note that should the District receive funds from other programs, these funds could be used to meet the amount required for local participation. The Rural Development Program also has a low interest loan program for applicants to use to meet their match.

The Rural Development programs require that funds not be released to the applicant until a construction contract is entered into. Although engineering fees are eligible for grant funds, interim payments from the grant funds are not allowed. These fees would include the cost of a preliminary engineering report required for the program as well as the basic design fees. Design drawings and specifications must be accepted by Rural Development prior to bid. The bid package must incorporate standard forms and terminology required by the program. Typically, the process of applying for and receiving a grant from the Department of Agriculture, Rural Development, takes a long time requiring between 24 and 30 months from submittal to approval for bid.

These grants are favorable for improvement of low income area infrastructure. New high density development would not be favored for these types of grants.

9.2 Economic Development Administration

The Economic Development Administration is part of the U. S. Department of Commerce. The purpose of this organization is to promote business growth and thus provide jobs for a service area. Grants for this program require written commitments from potential employers that will move to a service area if services are provided or statements from existing employers that they will move out if the services are not provided. The number of new jobs which can be attributed to the completion of the project is an important consideration for grants from this agency. Grants for this program typically require a 20 to 30 percent match from the applicant.

Processing time for the Economic Development Administration grant is typically between 12 and 18 months from submittal to approval for bid. Interim costs prior to construction will be paid as part of the process.

This approach could be utilized as commercial development increases along the IH-10 corridor.

9.3 Impact Fees from Prospective Developers

Special Utility Districts may develop and institute an impact fee program to share the costs of providing infrastructure improvements to their service area. The operation of such a program is governed by state laws and must be adopted by the Texas Commission on Environmental Quality.

In this approach, a master plan for infrastructure improvements for a projected population at some time in the future and the associated costs for these improvements is developed. These costs are prorated to the total projected number of services, and per service costs assigned to each new service. As construction on lots for new developments begin, the impact fees for that lot are paid. Existing developments are not required to pay these impact fees.

The establishment of an impact fee program allows GVSUD to have the costs of infrastructure improvements to be partially borne by new development. Funds collected from new development are assigned to an audited account and then used to construct the proposed improvements, as they are required. GVSUD shall establish a wastewater impact fee subcommittee to manage the program.

It is extremely important that GVSUD establish a wastewater impact fee for new development. The GVSUD strategy to develop wastewater service through new development requires GVSUD to have Impact Fee policy in-place.

9.4 Municipal Bonds

The District may issue revenue bonds for the financing of the proposed improvements based on the collection of future income from the project. The bonds would be issued based on the estimated costs of the proposed improvements in addition to associated issuance costs. Bonds are typically sold to Government agencies Federal — Rural Development Assistance (RDA), State of Texas Water Development Board (TWDB) and Private Bond Market. Interest rates and terms vary based on associated risk, taxable or tax-exempt issuance and lender. Bonds typically are 20-30 years in duration with a "call" period, minimum finance period of 10 years. Rates are typically 1 to 4 points over-prime lending rates. Presently these rates are 5-7%.

The bonds could be established through three different mechanisms. First, GVSUD could sell bonds based on the income from the water service revenues. This could be a political issue due to some water rate payers would not be benefiting from wastewater services, but are charged the rate of the bonds on their water bill. Second, GVSUD could establish its wastewater service area as a special taxing unit. This approach would require passed legislation for the special taxing entity. Again with this approach, the political turmoil would exist from some tax payers not benefiting from wastewater services.

Third, GVSUD would establish itself in the wastewater business by first taking ownership and operation of the Harvest Hills wastewater treatment plant. This would start revenues flowing into GVSUD. As additional developments come on line, increased revenues could assist GVSUD to sell bonds for the capital required to install wastewater infrastructure to connect the new developed areas. The infrastructure increase would grow at the same pace as development and would slowly lead itself to more desirable regional wastewater collection systems and treatment facilities.

9.5 USDA Rural Development, TWDB, or Co Bank

As GVSUD develops a wastewater customer base, further opportunities for the third-party debt financing option will materialize. A USDA Rural Development loan for wastewater infrastructure under 7 USCA §1926 will provide GVSUD with CCN protection from competing wastewater providers.

10.0 Proposed Impact Fees

Communities as well as utility districts may develop and institute an impact fee program. The impact fee allows developers to share the costs of providing infrastructure improvements to their area. Wastewater impact fees and rates for local wastewater service providers are presented below.

New Braunfels Utilities charges a wastewater impact fee and a sewer tap fee. The wastewater impact fee is \$1,160 per connection and the sewer connection fee is \$655 per tap.

The City of Seguin also charges a sewer impact fee and a sewer tap fee. The sewer impact fee is \$500 and the sewer tap fee is \$470.

Cibolo Creek Municipal Authority (CCMA), who offers wholesale wastewater treatment to areas in the Green Valley SUD, charges a sewer impact fee of \$985 per EDU (equivalent dwelling unit). The cost of treatment is \$1.60 per 1,000 gallons.

GBRA charges a monthly service fee as well as a connection fee for the wastewater service it provides. The connection fee is \$1,000 per EDU and the monthly service fee is \$32.

Green Valley Special Utility District Wastewater impact Fees and Rates Neighboring Utilities									
Neighboring Utility	Wastewater Impact Fee	Wastewater Connection Fee	Cost of Treatment (\$/1000 gal)	Monthly Service Fee					
New Braunfels Utilities	\$1,160	\$655							
City of Seguin	\$500	\$470							
CCMA	\$985		\$1.60						
GBRA		\$1,000		\$32					

The surrounding wastewater providers approach impact fees differently. These rates for the surrounding entities are considered normal for area developers. GVSUD should establish an impact fee rate schedule that benefits GVSUD and remains within the range of the surrounding wastewater providers. Further, GVSUD should consult with an accountant to establish the required rate necessary for GVSUD to recover the proper amount of capital.

To evaluate feasibility and determine probable impact fees, the total project cost of each drainage area was divided by the expected number of EDU's in each respective drainage area. It should be noted that these costs do not include any costs associated with operation and maintenance of the wastewater treatment facilities or service lines from customers to the collection system.

	Green Valley Special Utility District Wastewater Impact Fees Main Wastewater Collection System (Trunk Lines)											
Total Total Dev Density of 1 EDU/acre Dev Density of 3 EDU/acre												
Drainage	LUE	LUE	Total	Potential	Total	Potential						
Basin	1	3	Costs	Impact	Costs	Impact						
	(EDU/acre)	(EDU/acre)		Fee		Fee						
Drainage Basin A	9,211	27,633	\$11,212,950	\$1,217	\$13,229,734	\$479						
Drainage Basin B	4,690	14,070	\$3,379,449	\$721	\$3,848,841	\$274						
Drainage Basin C	5,636	16,908	\$4,151,280	\$737	\$4,773,440	\$282						
Drainage Basin D	6,688	20,064	\$3,072,068	\$459	\$4,188,876	\$209						
Drainage Basin E	35,618	106,854	\$34,601,813	\$971	\$43,682,177	\$409						
Drainage Basin F	6,515	19,545	\$5,230,109	\$803	\$6,739,925	\$345						
Drainage Basin G	7,511	22,533	\$3,963,086	\$528	\$4,673,334	\$207						
Total	75,869	227,607	\$65,610,755	\$865	\$81,136,327	\$356						

	Green Valley Special Utility District Wastewater Impact Fees Wastewater Treatment Facility											
	Total	Total	Dev Density of	1 EDU/acre	Dev Density of 3	EDU/acre						
Drainage	LUE	LUE	Total	Potential	Total	Potential						
Basin	1	3	Costs	Impact	Costs	Impact						
	(EDU/acre)	(EDU/acre)		Fee		Fee						
Drainage Basin A	9,211	27,633	\$7,898,433	\$858	\$20,310,255	\$735						
Drainage Basin B	4,690	14,070	\$4,021,675	\$858	\$12,065,025	\$858						
Drainage Basin C	5,636	16,908	\$4,832,870	\$858	\$14,498,610	\$858						
Drainage Basin D	6,688	20,064	\$5,734,103	\$857	\$17,202,308	\$857						
Drainage Basin E	35,618	106,854	\$26,179,965	\$735	\$39,269,948	\$368						
Drainage Basin F	6,515	19,545	\$5,586,613	\$858	\$16,759,838	\$858						
Drainage Basin G	7,511	22,533	\$6,440,683	\$858	\$16,561,755	\$735						
Total	75,869	227,607	\$60,694,342	\$800	\$136,667,739	\$600						

The above two tables estimate the impact fee range that GVSUD may need to require for new wastewater development. The top table represents potential impact fees associated with the main wastewater collection system, the bottom table shows potential impact fees associated with construction of wastewater treatment facilities.

The following table is a summary of the above two tables combined.

V	Green Valley Special Utility District Wastewater Impact Fee Summary Wastewater Collection and Treatment Combined Dev Density of 1 EDU/acre Dev Density of 3 EDU/acre											
Dev D Drainage Wastewater Basin Collection Impact fee		Density of 1 ED Wastewater Treatment Impact fee	atment Total		Wastewater Collection Impact fee	Wastewater Treatment Impact fee	Total Impact Fee					
Drainage Basin A	\$1,217	\$858	\$	2,075	\$479	\$735	\$1,214					
Drainage Basin B	\$721	\$858	\$	1,578	\$274	\$858	\$1,131					
Drainage Basin C	\$737	\$858	\$	1,594	\$282	\$858	\$1,140					
Drainage Basin D	\$459	\$857	\$	1,317	\$209	\$857	\$1,066					
Drainage Basin E	\$971	\$735	\$	1,706	\$409	\$368	\$776					
	\$803	\$858	\$	1,660	\$345	\$858	\$1,202					
Drainage Basin F Drainage Basin G		\$858	\$	1,385	\$207	\$735	\$942					

From the above impact fee summary, GVSUD may need to charge a wastewater impact fee of approximately \$2000 per EDU.

11.0 Recommendations

As previously developed, the cost of facilities to provide centralized wastewater service is less than or comparable to that provided by individual private on-site septic disposal systems. The benefits of public health and safety, water quality, both surface and groundwater protections are clearly evident.

The attached list of items are action items for consideration and implementation by the GVSUD Board of Directors.

- 1. Assume ownership and operation of Harvest Hills Property wastewater treatment plant.
- 2. Explore partnership opportunities with the City of Marion.
- 3. Aggressively pursue potential wastewater collection and treatment projects.
- 4. Limit Indendently Owner Utility (IOU) systems.
- Adopt design criteria standards.
- 6. Adopt Impact Fees.
- 7. Identify develop density to stay consistent across entire GVSUD wastewater CCN.
- 8. GVSUD Attorney Mr. Mark Zeppa has recommended amending GVSUD by-laws to clearly delineate its ability to provide wastewater service to its customers. Mr. Zeppa has suggested draft rules changes for adoption.
- 9. Establish wastewater management team as shown in the below hierarchy diagram.
- 10. Establish GVSUD wastewater subcommittee.
- 11. Establish and adopt official GVSUD wastewater policies, tariffs, and by-laws.

- 12. Establish contract with Harvest Hills as wastewater owner and operator. GVSUD shall develop a service plan to provide wastewater service to this tract and possibly adjacent tracts.
- 13. Revise service applications (standard and non-standard) and easement applications forms to reflect not only water but wastewater as well.
- 14. Establish sales and marketing strategy for wastewater services.
- 15.GVSUD should meet with City of Santa Clara officials to discuss wastewater service plans. The above No. 5 Harvest Hills tract is in Santa Clara's political limits. To prevent future wastewater utilities from entering GVSUD wastewater CCN service area, a development plan to resolve these issues should be explored.
- 16. The City of Marion has expressed a desire to provide wastewater service outside its city limits. Discussions should be held to formalize a service plan between GVSUD and the City of Marion to insure infrastructure for wastewater service.
- 17. Discussion with GBRA on further regional long range wastewater facilities of much larger scale. Future wastewater needs for the GBRA CCN located to the Northeast of GVSUD's CCN will exceed current site capacity and a new larger site will be required.
- 18. New developments will fund wastewater systems with new construction. GVSUD should look for possible grants and innovative funding options to provide centralized service to existing subdivisions and developments. This would allow retrofitting and abandonment of their onsite systems for conversion to a centralized system. This can occur as service plans are developed.
- 19. Discuss with the residents of Treasure Island area the necessity to do away with existing individual below grade septic systems that could be potentially contaminating the Guadalupe River (Lake McQueeney). Propose GVSUD options to provide quality wastewater service to this area.
- 20. Adopt a formal development density to stay consistent throughout the GVSUD wastewater CCN service area.
- 21. Consider USDA Rural Development Assistance or TWDB type loans for CCN protection from competing wastewater providers who may attempt to take portions of GVSUD wastewater CCN service area.
- 22. Establish GVSUD wastewater design criteria standards.
- 23. Further study to implement wastewater impact fees.
- 24. Promote and advertise public meeting with development community.
- 25. Hire wastewater operator

River City Engineering, Ltd. is prepared to assist GVSUD with this long-term planning and assessment to implement this much needed utility service. Mutual cooperation with area utility systems and regulator authority will insure proper service and development.

12.0 Management Plan

GVSUD Board of Directors, General Manager, and Staff

For GVSUD to aggressively enter into the wastewater business there must be a wastewater team established. Due to the start-up nature of GVSUD involvement with wastewater, GVSUD needs internal motivation and aggressive wastewater board of directors, general manager, and staff. Included with duties of GVSUD should be the development of vision and mission statement to clearly define to its customers the long term wastewater goals.

GVSUD wastewater manager should continuously search for wastewater business opportunities to gain the competitive advantage with potential competition in the GVSUD wastewater CCN service area. GVSUD should organize wastewater subcommittee who can spend the time required to properly manage and get wastewater action items completed.

Engineering Consultant

GVSUD has hired River City Engineering, Ltd. (RCE) to develop this overall wastewater master plan. RCE is prepared to provide GVSUD technical direction beyond the adoption of this master plan and assist GVSUD into the wastewater business.

Legal Consultant

Mark Zeppa has been hired to represent GVSUD to establish formal policies and tariff rates. Also, Mr. Zeppa provides advice with general legal approach for GVSUD policy, rate structure and tariffs, rules and regulations, by-laws, and the eventual implementation of development impact fees.

Financial Consultant

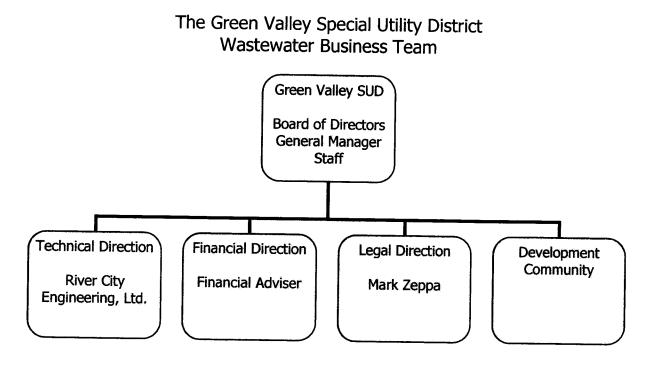
GVSUD has several options to consider for funding of wastewater projects. GVSUD needs to establish a capital budgeting procedure. The procedure should at a minimum define the process of project identification, evaluation, selection, and verification.

GVSUD should hire a financial adviser to assist with bond opportunities. The financial adviser shall assist GVSUD with capital through equity versus capital through debt, financial consultation and direction, and bond management.

GVSUD should also be aware of available grants and loans.

Development Community

The development community shall provide direction and assistance with initial wastewater start-ups dealing with new development. The area is prime for growth and the development community can assist bringing wastewater customers to GVSUD. There is a great deal of negotiation and dialog between GVSUD and the development community dealing with subjects like project phasing and cost assistance.

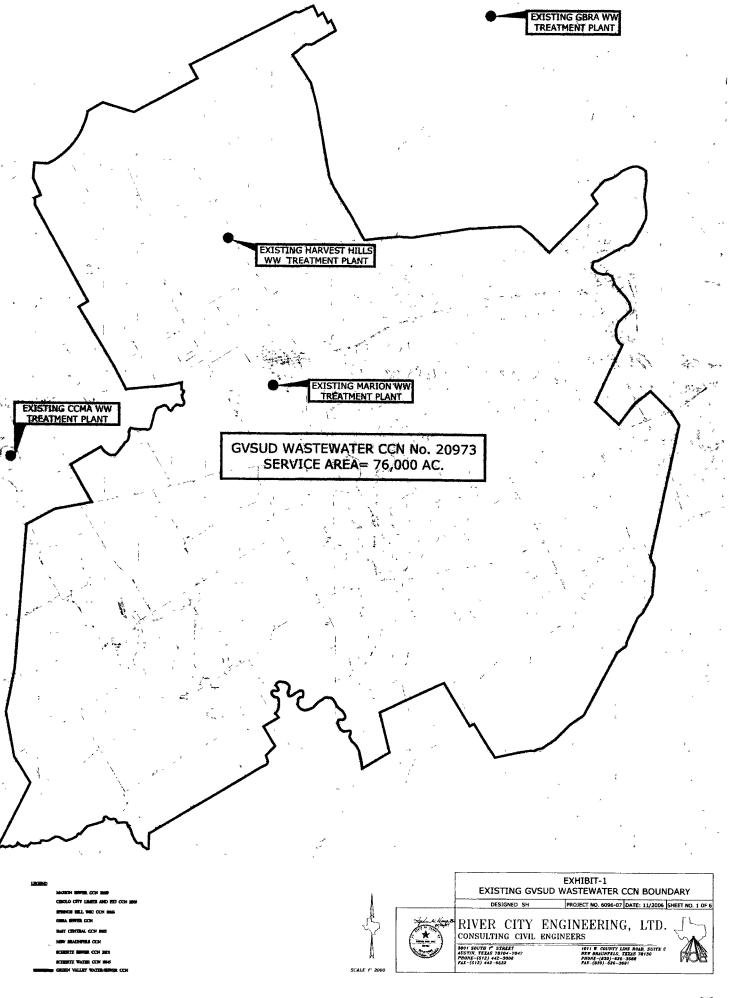


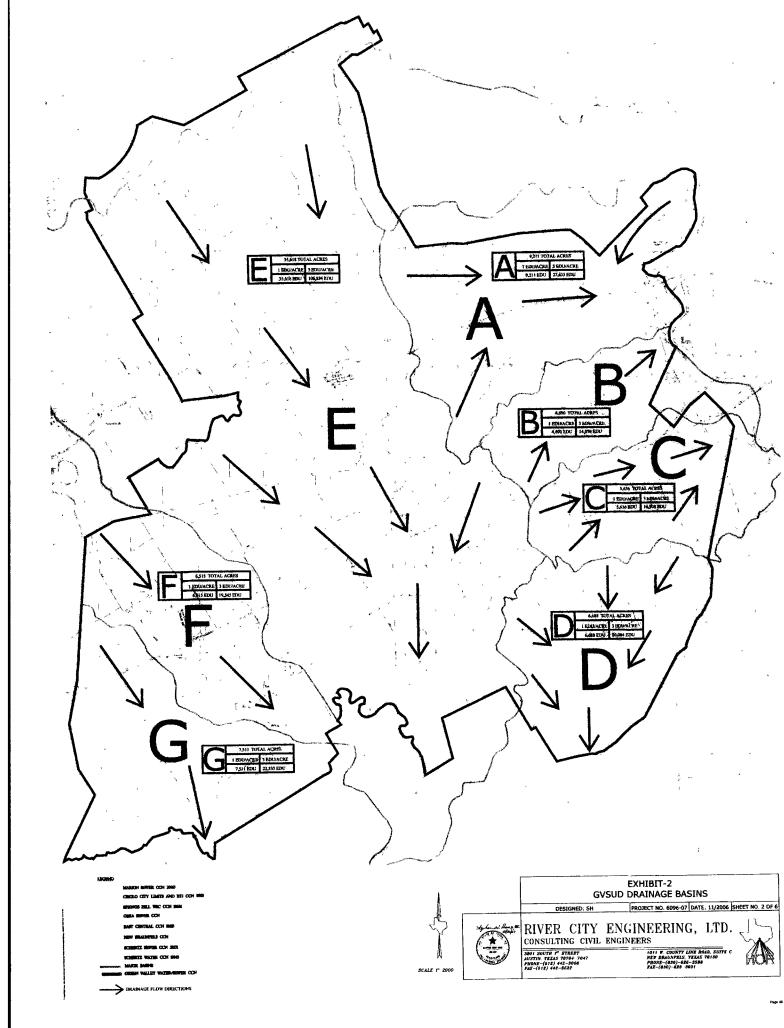
13.0 Conclusion

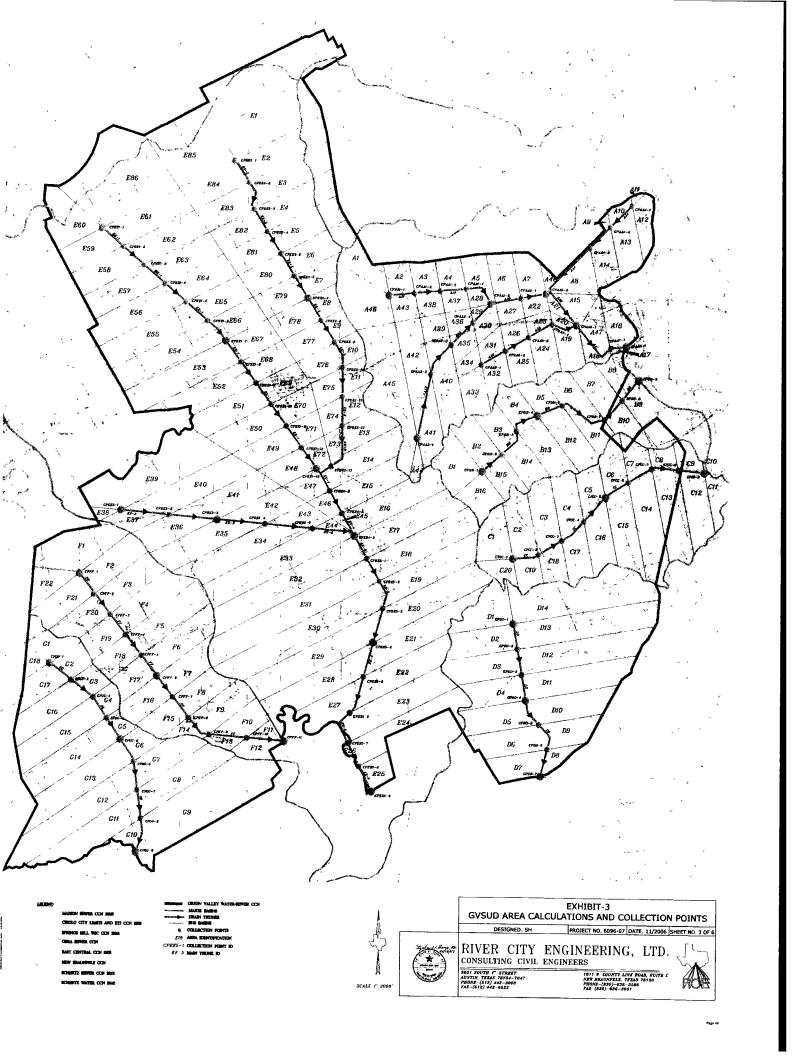
RCE looks forward to working with GVSUD with the wastewater venture in the future. The GVSUD wastewater CCN service area is prime for development and RCE recognizes GVSUD for the leadership and vision required to bring wastewater services to their customers. With continued support from the above wastewater team, GVSUD should prove itself to be the leader for quality wastewater service in the region.

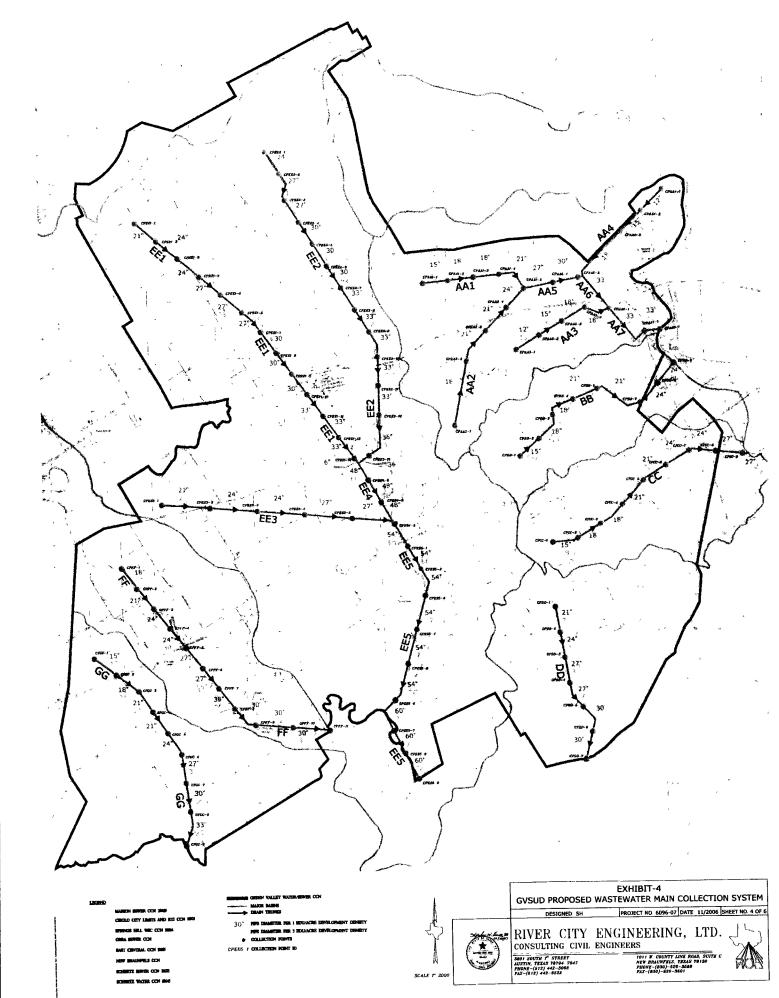
Attachment 1Exhibit Drawings

- Exhibit 1 GVSUD Existing Wastewater CCN No. 20973 Boundary
- Exhibit 2 GVSUD Drainage Basins
- Exhibit 3 GVSUD Area Calculations and Collection Points
- Exhibit 4 GVSUD Proposed Wastewater Main Collection System
- Exhibit 5 GVSUD Theoretical Locations of Wastewater Treatment Capacity and Costs vs. Downstream Locations
- Exhibit 6 GVSUD Proposed Growth and Development

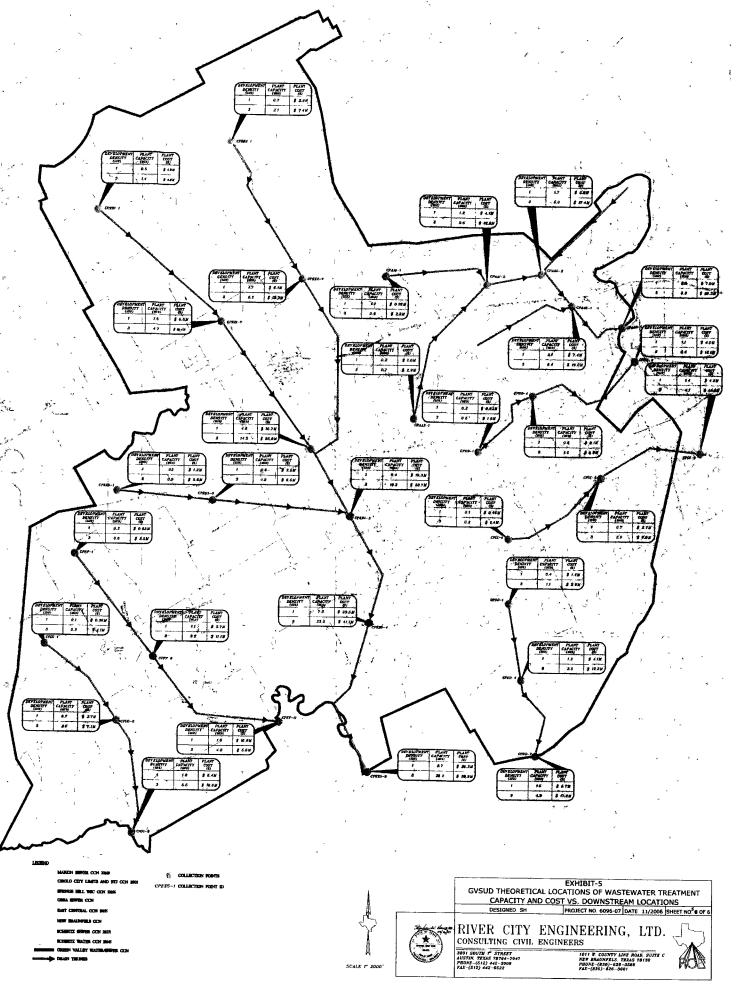




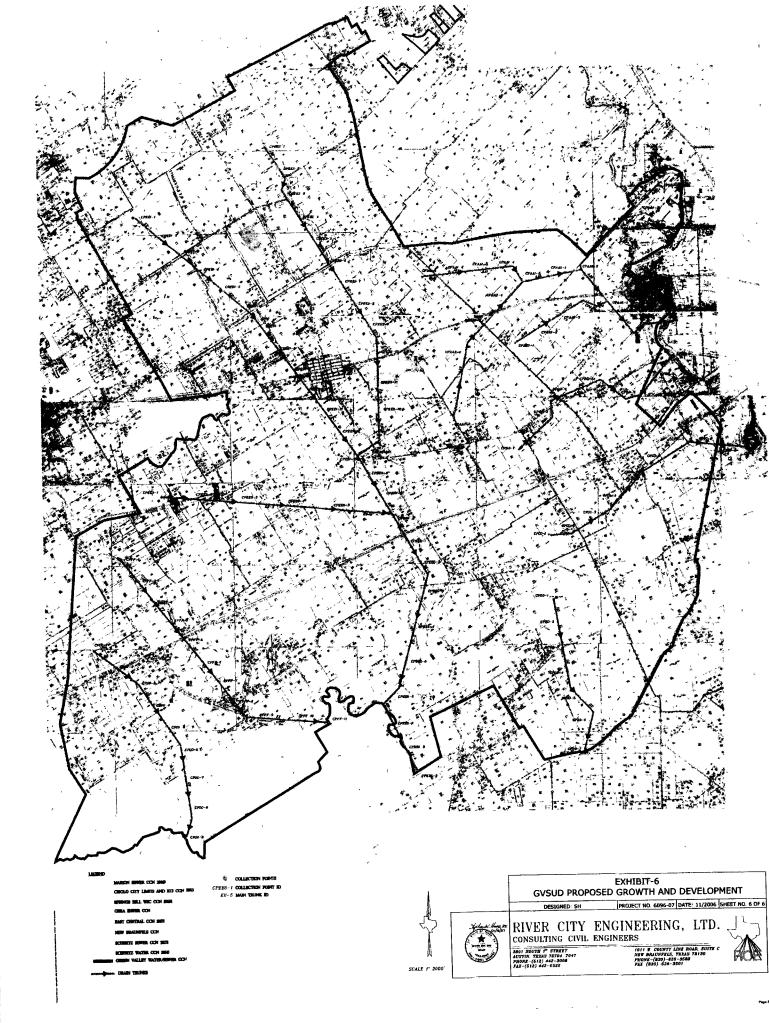




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Attachment 2Calculation Tables

- Exhibit 1 Total Equivalent Dwelling Unit (EDU) Calculations
- Exhibit 2 Wastewater Main Collection System Calculations
- Exhibit 3 Proposed Costs for Wastewater Main Collection System
- Exhibit 4 Wastewater Treatment Plant Capacity and Costs

	Green Valley Special Utility District EDU Calculation Summary											
Total EDU Calculations												
Drainage Total Development Total Development Total Basin Area Density EDU Density EDU												
ID	(acres)	(1 EDU/acre)	1.	(3 EDU/acre)	3							
Α	9,211	1	9,211	3	27,633							
В	4,690	1	4,690	3	14,070							
<u> </u>	5,636	1	5,636	3	16,908							
D	6,688	1	6,688	3	20,064							
E	35,618	1	35,618	3	106,854							
F F	6,515	1	6,515	3	19,545							
G	1 0,515 1 7,514 2 22,527											
	75,869		75,869		227,607							

Green Valley Special Utility District Drainage Area A Total FDU Calculations

A

			lotal EDU (Caicula	tions			
Sub-Area Left ID	Sub-Area Right ID	Sub-Area Left (acres)	Sub-Area Right (acres)	Total Area (acres)	Development Density (1 EDU/acre)	Total EDU	Development Density (3 EDU/acre)	Total EDU
Pipe AA1							1	
A1	A46	256	520	776	1	776	3	2,328
A2	A43	177	144	321	1	321	3	963
A3	A38	169	129	298	1	298	3	894
A4	A37	175	114	289	1	289	3	867
A5	A28	210	73	283	1	283	3	849
			Total Acres =	1967	Total EDU =	1,967	Total EDU =	5,901
Pipe AA 2								
A45	A44	931	71	1002	1	1,002	3	3,006
A42	A41	486	287	773	1	773	3	2,319
A39	A40	234	394	628	1	628	3	1,884
A36	A35	131	119	250	1	250	3	750
A29	A30	34	133	167	1	167	3	501
			Total Acres =	2820	Total EDU =	2,820	Total EDU =	8,460
Pipe AA3		·						
A34	A33	93	246	339	1	339	3	1,017
A31	A32	127	195	322	1	322	3	966
A26	A25	93	262	355	1	355	3	1,065
A23	A24	127	220	347	1	347	3	1,041
A20	A19	50	240	290	1	290	3	870
Pipe AA4			Total Acres =	1653	Total EDU =	1,653	Total EDU =	4,959
A12	A11	211	17	228	1	228		604
A13	A10	314	69	383	1	383	3	684
A14	A9	252	36	288	1	288		1,149
A8	A48	269	72	340	1	340	3	864
			Total Acres =	1239	Total EDU =	1,239	Total EDU =	1,021
Pipe AA5			7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7		TOTAL EDG =	1,239	TOTAL EDU =	3,718
A6	A27	233	103	336	1	336	3	1,008
A7	A22	266	118	384	1	384	3	
			Total Acres =	720	Total EDU =	720	Total EDU =	1,152 2,160
PipeAA6				720	TOWN EDG =	720	rotal EDU =	2,100
A15	A21	181	62	243	1	243	3	730
			Total Acres =	243	Total EDU =	243	Total EDU =	730
PipeAA7						273	TOWN EDU -	/30
A47	A18	208	92	301	1	301	3	902
			Total Acres =	301	Total EDU =	301	Total EDU =	902
PipeAA8						301	TOTAL EDG -	702
A16	A17	253	15	268	1	268	3	804
			Total Acres =	268	Total EDU =	268	Total EDU =	804
						200	I JULAI EDU -	004
			Basin A (acres) =	9,211	Basin A (EDU) =	9,211	Basin A (EDU) =	27.633

Green Valley Special Utility District Drainage Area B Total EDU Calculations

B

	Total EDO Calculacións											
Sub-Area Left ID	Sub-Area Right ID	Sub-Area Left (acres)	Sub-Area Right (acres)	Total Area (acres)	Development Density (1 EDU/acre)	Total EDU	Development Density (3 EDU/acre)	Total EDU				
	10	(46.55)										
Pipe BB	D16	367	267	634	1	634	3	1,903				
B1	B16			494	1	494	3	1,482				
B2	B15	287	207			480	3	1,439				
B3	B14	229	251	480	<u>L</u>		3	1,747				
B4	B13	216	366	582	1	582						
B5	B12	190	384	574	1	574	3	1,723				
	<u> </u>	285	206	491	1	491	3	1,474				
B6	B11			515	1	515	3	1,545				
B7	B10	306	209			920	1 - 2	2,759				
B8	B9	216	704	920	<u> </u>		Desir D (EDII) -					
			Basin B (acres) =	4690	Basin B (EDU) =	4,690	Basin B (EDU) =	14,0/1				

Green Valley Special Utility District Drainage Area C Total EDU Calculations Sub-Area Sub-Area Sub-Area Sub-Area Total **Development** Total Development Total Left Right Left Right Area **Density** EDU **Density EDU** ID ID (acres) (acres) (1 EDU/acre) (acres) (3 EDU/acre) Pipe CC C20 C1 314 150 464 1 464 1,391 C2 C19 365 173 537 1 537 3 1,612 **C3** C18 373 192 565 1 565 3 1,694 C4 C17 331 271 602 1 602 3 1,807 C5 C16 233 332 565 1 565 3 1,696 C6 C15 259 457 716 1 716 3 2,149 **C7** C14 203 584 788 1 788 3 2,363 <u>C8</u> C13 119 520 639 1 639 3 1,916 C9 C12 152 405 557 1 557 1,670 C10 C11 9 194 203 203 610 Basin C (acres) = 5636 Basin C (EDU) = 5,636 Basin C (EDU) = 16,908

Green Valley Special Utility District Drainage Area D Total EDU Calculations Total Development Development Total Total Sub-Area Sub-Area Sub-Area Sub-Area **EDU EDU Density Density** Area Right Right Left Left (3 EDU/acre) (1 EDU/acre) (acres) (acres) (acres) ID ID Pipe DD 4,405 1,468 1468 1 401 1068 D1 D14 3,567 3 1,189 1189 1 374 815 D13 D2 3,406 3 1,135 1 411 1135 725 D3 D12 2,809 3 936 1 326 936 610 D4 D11 2,527 3 842 842 1 376 466 D5 D10 2,071 3 690 690 1 407 283 D6 D9 1,277 426 426 297 128 D7 D8 Basin D (EDU) = 6,688 Basin D (EDU) = 20,063 Basin D (acres) = 6688

Green Valley Special Utility District Drainage Area E Total EDU Calculations Sub-Area Sub-Area Sub-Area Sub-Area Total Development Total Development Total Left Right Left Right Area **Density EDU Density EDU** ID (1 EDU/acre) ID (acres) (acres) (acres) (3 EDU/acre) Pipe EE 1 E86 E60 1455 442 1897 1 1,897 3 5,691 E61 E59 476 311 787 1 787 2,361 3 E62 E58 365 424 789 1 789 3 2,366 E63 E57 383 419 802 1 802 3 2,406 E64 E56 343 517 860 1 860 3 2,579 E65 E55 302 58 360 1 360 3 1,080 E66 E54 267 625 892 892 1 3 2,676 E67 E53 259 646 905 905 3 2,715 E68 E52 255 471 725 1 725 2,176 E69 E51 248 416 664 1 664 3 1,993 E70 E50 224 381 605 1 605 3 1,816 E71 E49 167 312 479 1 479 3 1,438 E72 E48 130 250 381 381 3 1,142 Total Acres = 10146 Total EDU = 10,146 Total EDU = 30,438 Pipe EE 2 E1 E85 2297 574 2871 2,871 8,612 E2 E84 519 347 866 1 866 3 2,598 **E3** E83 484 322 806 806 1 3 2,417 E4 E82 464 279 743 1 743 2,230 3 **E**5 E81 419 273 692 1 692 3 2,076 **E6** E80 406 266 673 1 673 3 2,018 E7 E79 229 260 489 1 489 3 1,466 E8 E78 151 253 404 1 404 3 1,213 **E**9 E77 135 247 382 1 382 3 1,146 E10 E76 142 232 374 374 3 1,122 E11 E75 161 183 345 1 345 1,034 E12 F74 151 140 291 291 1 3 874 E13 E73 291 110 401 401 1,204 Total Acres = 9337 Total EDU = 9,337 Total EDU = 28,011 Pipe EE 3 E39 E38 1168 50 1218 1 1,218 3,655 F40 E37 619 179 797 1 3 797 2,392 E41 E36 477 32 509 509 3 1,526 E42 E35 334 509 843 1 843 3 2,530 E43 F34 192 628 820 820 1 3 2,460 E44 E33 51 805 856 856 2,568 Total Acres = 5044 Total EDU = 5,044 Total EDU = 15,132 Pipe EE 4 E14 E47 466 184 649 649 1,948 E15 E46 414 106 520 520 3 1,560 E16 E45 578 28 606 606 1,817 Total Acres = 1775 Total EDU = 1,775 Total EDU = 5,325 Pipe EE 5 E17 E32 607 925 1532 1 1,532 3 4,596 E18 E31 644 1036 1679 1,679 5,038 1 3 E19 E30 585 1003 1588 1,588 4,764 E20 F29 67 801 868 1 868 3 2,603 E21 E28 621 650 1271 1 1,271 3 3,813 E22 E27 567 375 942 1 942 3 2,825 E23 E26 665 17 682 1 682 3 2,045 E24 564 564 564 3 1,692 E25 191 191 191 574 Total Acres = 9317 Total EDU = 9,317 Total EDU = 27,950 Basin E (acres) = 35,618 Basin E (EDU) = 35,618 Basin E (EDU) = 106,855

Green Valley Special Utility District Drainage Area F Total EDU Calculations

F

	lotal EDU Calculations											
Sub-Area Left ID	Sub-Area Right ID	Sub-Area Left (acres)	Sub-Area Right (acres)	Total Area (acres)	Development Density (1 EDU/acre)	Total EDU	Development Density (3 EDU/acre)	Total EDU				
	72 Sept. 19							***************************************				
Pipe FF	and the second second					0.47		2 542				
F1	F22	614	233	847	1	847	3	2,542				
F2	F21	379	343	722	1	722	3	2,166				
F3	F20	389	291	680	1	680	3	2,040				
F4	F19	451	229	680	1	680	3	2,041				
F5	F18	500	239	739	1	739	3	2,216				
	F17	452	229	681	1	681	3	2,042				
F6		313	284	597	1	597	3	1,791				
F7	F16			528	1	528	3	1,585				
F8	F15	295	233		1	451	3	1,354				
F9	F14	363	89	451	1		3	1,010				
F10	F13	250	86	337	1	337	3					
F11	F12	82	171	253	1	253	3	758				
			Basin F (acres) =	6515	Basin F (EDU) =	6,515	Basin F (EDU) =	19,544				

Green Valley Special Utility District Drainage Area G Total EDU Calculations

G

- otti 120 caicaiations											
Sub-Area Left ID	Sub-Area Right ID	Sub-Area Left (acres)	Sub-Area Right (acres)	Total Area (acres)	Development Density (1 EDU/acre)	Total EDU	Development Density (3 EDU/acre)	Total EDU			
A CONTRACTOR OF THE PARTY OF TH	Sec. 18 19 19	1000		Andreas (Company)			And the second				
Pipe GG			-				Committee to the second	Section Control			
G1	G18	267	96	363	1	363	3	1,088			
G2	G17	187	268	455	1	455	3	1,365			
G3	G16	144	452	596	1	596	3	1,789			
G4	G15	129	552	681	1	681	3	2,043			
G5	G14	90	594	685	1	685	3	2,054			
G6	G13	160	661	821	1	821	3	2,463			
G7	G12	387	838	1225	1	1,225	3	3,674			
G8	G11	563	600	1163	1	1,163	3	3,488			
G9	G10	1410	113	1523	1	1,523	3	4,569			
			Basin G (acres) =	7511	Basin G (EDU) =	7,511	Basin G (EDU) =	22,534			

Green Valley S Design F	low Summar	у
Design Flow	Developme	nt Density
J	1 EDU/Acre	3 EDU/Acre
Average Dry Weather Flow	245 GPD/EDU	245 GPD/EDU
Maximum Dry Weatherh Flow	735 GPD/EDU	735 GPD/EDU
Maximum Wet Weather Flow	1485 GPD/EDU	985 GPD/EDU

Green Valley Special Utility District
Wastewater Design Flows
Three Design Flow Conditions

				Developm	ent Density of	1 EDU/acre	Developm	ent Density of	B EDU/acre
Drainage Basin	Total Area (acres)	Totai EDU 1 (EDU/acre)	Total EDU 3 (EDU/acre)	Average Dry Weather Flow (GPD)	Maximum Dry Weather Flow (GPD)	Maximum Wet Weather Flow (GPD)	Average Dry Weather Flow (GPD)	Maximum Dry Weather Flow (GPD)	Maximum Wet Weather Flow (GPD)
Drainage Basin A	9,211	9,211	27,633	2,256,695	6,770,085	13,678,335	6,770,085	20,310,255	27,218,505
Drainage Basin B	4,690	4,690	14,070	1,149,050	3,447,150	6,964,650	3,447,150	10,341,450	13,858,950
Drainage Basin C	5,636	5,636	16,908	1,380,820	4,142,460	8,369,460	4,142,460	12,427,380	16,654,380
Drainage Basin D	6,688	6,688	20,064	1,638,560	4,915,680	9,931,680	4,915,680	14,747,040	19,763,040
Drainage Basin E	35,618	35,618	106,854	8,726,410	26,179,230	52,892,730	26,179,230	78,537,690	105,251,190
Drainage Basin F	6,515	6,515	19,545	1,596,175	4,788,525	9,674,775	4,788,525	14,365,575	19,251,825
Drainage Basin G	7,511	7,511	22,533	1,840,195	5,520,585	11,153,835	5,520,585	16,561,755	22,195,005

	(Green \			Utility I	District	•	
				DU/a				1
		Pipe I			gn Sum			
Pipe	Basin A	Basin B	Basin C	Basin D	Basin E	Basin F	Basin G	Total
Diameter	Pipe	Pipe	Pipe	Pipe	Pipe	Pipe	Pipe	Pipe
	Length	Length	Length	Length	Length	Length	Length	Length
(in)	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)
8								0
10	-							0
12	5,600							5,600
15	8,200	2,600	2,600				2,700	16,100
18	19,800	5,600	5,600			2,500	2,700	36,200
21	9,000	7,000	5,600	2,500	7,350	2,500	5,000	38,950
24	3,500	5,000	5,200	2,500	17,200	5,000	2,500	40,900
27	2,500		5,000	5,300	22,300	5,000	2,800	42,900
30	2,500			5,600	15,450	12,200	2,900	38,650
33	11,400				21,650		3,000	36,050
36	-				5,800			5,800
42								0
48					7,000			7,000
54			1		18,500			18,500
60	+				9,600			9,600
66	 							0
72								0
Total	62,500	20,200	24,000	15,900	124,850	27,200	21,600	296,250

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Manning's Roughness Coefficient = Percent of Pipe Flowing Full =

GPD/EDU GPD/EDU GPD/EDU

245 735 1485

Average Dry Weather Flow = Maximum Dry Weather Flow = Maximum Wet Weather Flow =

gallon/acres served

GPD capita/EDU EDU/acre GPD/capita

245 3.5 3.7 750 750

Residential Single Family Units (EDU) =
Population per EDU =
Development Average Density =
Wastewater Demand =
Maximum Flow Peak Factor =
Inflow/Inflitration =

d	T	Ţ	Pipe Velocity	(ft/sec)	Ī	60	29	4.62	29	П	1	,	10	4.67	5.11	П		Ţ	3 :	200	ະ	П	П	č	1 2	-	П	1		2	2	Ţ	Τ		Ţ	T	Τ		_	T	Τ	3	Π
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~		2	Pipe Diameter	(Inches)		14.50	15.87	17.03	18.68			17 07	20.14	20.90	21.38			11 56	13.81	15.53	16.02			12.05	13.93	15.71				26.75	27.48				30.05	İ			32.96			30.89	
	e September	0	Pipe Diameter Actual	(inches)		11.60	12.70	13.62	4. R			14 38	16.11	16.72	17.10			20.0	11.05	12.42	12.82			29.0	11.14	12.57				21.40	21.98				2.04	T			26.37	1		24.71	
	Pine Decim	2	Pipe	Œ		09000	0.0060	09000	0.0050	0.0058		0.000	0.0050	0.0050	0.0050	0.0050		0.0072	0.0067	0.0064	0.0080	0.0071		0.0050	0.0050	0.0050	0.0050		ľ	0.0050	0.0050				0.0050				0.0050	2000		0.0075	,0075
	l		Pipe Length	(¥)		2,500	2,590	2,500	-	11,000		Т	Т		_	17,500		+			_	10,600		7	3,000		12,000	1	Ī	\vdash	2,500	_		-	4,500	_			4,900	_		2,000	_
		9	Downstream Invert Elevation	£		585	570	555	1			578	565	553	T			581	583	545	7				536					535	T	T			SZ	T		1	210	T		495	
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		14	the	1	1.78	2.52	3.21	3.8/	75.7		2.30	4.08	5.52	6.10	0,48	1	0.78	1.52	2.33	3.13	3.80	†	65.0	1.40	2.07	2.85	1	4.50	11.00	11.77	17.05		2.85	15.50	3		3.80	19.86	6.5	f	20.55	21.16	
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ĸ	Maximum Wet Weather	1		4	1.78	+	+	0000	\pm	+	+	1.78		0.57	+	╀	╀	0.74	Н	0.80	+	1	+	Н	0.66	+	╀	╀	-	-	12.65	Н	+	12.65	╁	Н	3.80	+	╁	┞	20.55	+	1
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i odiny bistr A - 1 EDU Calculations		Н	5		1,152,30	4/6,065	420 165	420 255	2 020 00	2/326/3	1.487.97	1,147,905	932,580	371,250	4.187.700		503,415	478,170	527,175	515,295	2 454 70E	2/402/7	338,580	568,755	427,680	200,385	7,001	2,920,995	4,187,700	498,960	8,177,895		1,841,400	360,855	10,380,150		2,454,705	445 500	13,280,35		13,280,355	12 676 22	edia min
zen vaney special Ounty District Drainage Area A - 1 EDU Ape Flow Design Calculations	rage Dry Weather Flow	10	Cumulative Dry Weather Flow	(a)	150,120	241 775	412 580	481 915	22,020		245,490	434,875	588,735	600,000	200,000		83,055	161,945	248,920	333,935	COCYLOT		55,860	149,695	220,255	202,000		481,915	1,172,815	1,255,135	40.12/240		303,800	1,712,550			3 117 535	2,117,535	200120212		2,191,035	1	
Drain	Average Dry V	6	Veather Flow	Q49	130,120	73,010	70.805	69.335	81.915		245,490	189,385	153,860	40 915	90.900		83,055	78,890	86,975	21,050	04.985		55,860	93,835	70,560	2000		81,915	006'069	82,320	1,349,215		340 245	59,535	1,712,550	\mathbb{H}	1 712 550	+	╀	H	1,035	1605	
	H	H		(EDU)	+	288	╄	┾	╄	╄	H	Н	+	167	1	┡	H		4	78/	╀	╀	-	4	8 2	1	+		-	384	-	_	-	243	_	+	1653	+-	-	\vdash	368 2,	11 2.25	1
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	wer Main	7	Stream Collection	1 Ubstre	8	1 CP AA1-2	₽ G	4 A	Tota		2 Upstream	₩	100	O AA24	Tota		8 Upstream	C Wall	3 6	834	Tota		Upstream	3 6	Q A44-3	Total		Upstream	Sarres Sarres	Q A85-1	Total	1	O AA:	CP AAS-2	Total	Inctres	CP AA5-2	CP AA6	Total	988	84.54 847:1-1		
	Ź	-	<u>e</u>	Ploe AA	Pipe AA1	Pipe AA1	Pipe AA	Pipe AA:	_		Pipe AA2	Pipe AA.	A Par	Pipe AA2	2		Pipe AA3	Tipe AA	The And	Pipe AA3	_		Pipe AA4	TOP AA4	Pipe AA4	4		Ppe AA1	De Akz	Pipe AA5	2	AND AND	Pipe AAS	ipe AA6	9	Pine AA3	De AA6	Pipe AA7	/	Dine AA7	Pipe AA8	8	

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סם	;			Nominal	(inches)		12	2	2	7	77	7	7		
1 EDU	١١	1	Nameter Manuele	80% Full	(inches)		13.99	16.20	18.19	19.57	21.29	21.83	23.69		
	uō,	13	2	Actual	(inches)		11.19	12.96	14.55	15.66	17.03	17.46	18.95		
	121	-1	2 2		E		0.0077	0.0071	0.0071	0.0077	0.0068	0.0000	0.0000	0.0076	
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		25	Upetream E	Invert	(H)	†	9		3	3	200	200	200		
	3	*	Cumulative	Wet Weather	MGE 3	Į.	200	2.00	3.03	2.00	460	QP.)	97.07	10,70	
	Ons Haximum Wet Weather	43	Wet	Weather	<u>}</u>	3	P :		1:10	\$ 1	1.32	217	1.16	7117	36.70
strict	Ons	Ş	ž	Weather	HOM		2	Š	\$	8	265	ŝ	2	- 1	2
Utility D 8 - 1 EDU		;	13	Weather	How	(GLS)	941,490	733,590	712,800	864,270	852,390	729,135	764,775	1,366,200	6,964,650
Green Valley Special Utility District Drainage Area B - 1 EDU	Pipe Flow Design Calculations	The sale and the sale and	Companie	Dry Weather	How	(GPD)	155,330	276,360	393,960	536,550	677,180	797,475	923,650	1,149,050	
Green Val	Pipe Fi	VAC BOOK	• [Weather	Flow	(GMD)	155,330	121,030	117,600	142,590	140,630	120,295	126,175	225,400	1,149,050
		8	8	1			534	ĝ	8	582	574	491	515	920	4690
		Homemdod		Development 10th		(1 EDU/acre) (EDU)	2	6.1	1.0	97	7.0	1.0	1.0	0,1	
		Area	•		8	(acres)	Ş	\$	8	3	2	8	515	8	4690
		Contributing Area	8	Sight.	A B B B	3	╁	782	200	3,5	5	¥	3	+	H
		3	4	5				+	+	+	+	┿	+	+	H
		ocation	~	Down	Stream Stream	Pedal	ľ	+				+	+	╁	
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		3	-	94	2		1	200	8	90	2	2		2	

Manning's Roughness Coefficient = Percent of Pipe Flowing Full =
GPD/EDU GPD/EDU GPD/EDU
245 735 1485
Average Dry Weather Flow = Maximum Dry Weather Flow = Maximum Wet Weather Flow =
GPD capita/EDU EDU/acre GPD/capita gallon/acres served
245 3.5 1 70 3 750
Design Parameters: Residential Single Family Units (EDU) = Population per EDU = Development Average Density = Wastewater Dennand = Maximum Flow Peak Factor = Inflow/Inflitration =

Green Valley Special Utility District	Drainage Area C - 1 EDU	Pipe Flow Design Calculations

								Drai	Drainage Area C - 1 EDU	C-1ED	-									2	C
								Pipe Fi	Pipe Flow Design Calculations	Calculat	ions								1	2)
Sex	Sewer Main Location	ocation	Con	Contributing Area	Les	Population	۽	Average Dry	orage Dry Weather Flow		Maximur	Maximum Wet Weather	Her								
-	7	m	•	2	9	,		6	9	-	÷	ç	1	ļ	ŀ		ripe vesign	ugu			
Pipe	9	Down	Ę9	Right	Total	Develonment Total	į	1				3			Io	77	18	19	20	77	77
£			1	į				5			Mer	¥	Cumulative	Upstream	Downstream	Pipe	Pine	age d	arig d	2	o io
3	Henc	Eleope	o de	20 S	8	Density	2	Weather	Dry Weather	Weather	Weather	r Weather	Wet Weather	Invert	Troort	-	Slone	No.		2	2
	Collection	Collection	Area	Area				Flow	How	How	How	-		_	- Controller			Januar		Mameter	Verocity
	Point	Point	(acres)	(acres)	(agres)	(acres) (1 EDU/acre) (EDU)	(EDU)	(005)	(000)	(Cap)		1	1	ione and	Crevation	-		_	80% Full	Nominal	Actual
Dine	Jinchroam	9	٤	Š	ļ		ļ					(2)	(CIS)	Έ)	Œ	E	E		(inches)	(inches)	(ft/sec)
	1	3	3	-170	Ē	0.1	ŧ	113,660	113,680	689,040	479	1.07	1.07			l	L.	ł	İ	ł	
	9 C-1		173	365	238	0.1	238	131.810	245.490	708 930	נננ	PC 1	2.0	200	207	300	-				
Pe CC	2-55 C)	C-55-0-	192	373	565	-	בעצ	128 475	202 015	200,000	66	1307	6.30	3	260	7,000	0.00//	10,70	13.38	15	4.63
Pipe CC	ر- <u>ن</u>	45.0	177	121	503		3 8	47.400	203,943	033/023	283	1.30	3.00	280	260	2,800	0.0071	12.83	16.04	18	5.04
0	1	3 6	1,7		7 1	2,5	3	14/430	551,405	893,970	621	1.38	4.98	260	240	2,800	0.0071	14.50	18.12	œ	100
		3 8	700	553	202	0,1	Š	138,425	669,830	839,025	583	1.30	6.28	540	220	2,800	0.0071	15.91	10.77	2	5 6
2	ı	ჭ ზ	457	526	716	27	716	175,420	845.250	1.063.260	738	1 65	7 03	220			7 (00.0	10.01	13.77	77	2.58
De CC	9 0 0	965	584	203	787	1.0	787	192.815	1 038 065	1 168 605	613			250	200	200/7	0.00/1	1/.65	21.57	21	5.58
Pipe CC	C-CC-2	9 5 8	520	119	639	6	029	156 555	1 104 620	240 040	210	101	2,72	36	980	2,600	0.0077	18.38	22.97	74	6.33
Pipe CC	8-55 G	6-50-60	5	157	757	-	6	136 ACE	1 221 005	270,513	9	7:4	11.20	480	460	2,600	0.0077	19.37	24.22	74	6,33
Pipe CC	ľ	Q CC-10	2	0	ě		Š	40 725	1 360 630	Ch1/70	4/6	1.28	12.48	460	5	2,500	0.0080	20.03	25.03	27	6.99
	TELO L		2578	2250	EGSE			000 000	I	201,133	507	0.4/	12.95	440	420	2,500	0.0080	20.31	25.38	27	6.99
					200		2020	1,350,520		8,369,460	5.812	12.95				A AAA	2400		İ		

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Residential Single Family Units (EDU) = 245 GPD Population per EDU = 3.5 capita/EDU Development Average Density = 1 EDU/acre Wastewater Demand = 70 GPD/capita Maximum Flow Peak Factor = 3 calion/acres served	Average Dry Weather Flow = 245 Maximum Dry Weather Flow = 735 Maximum Wet Weather Flow = 1485	
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Manning's Roughness Coefficient = Percent of Pipe Flowing Full =

GPD/EDU GPD/EDU GPD/EDU

۵		22	90,00	2) The second	(4/ear)			4.01	4.39	4.64	4.83	4.97	5.05			
2		-		_	_	_	1		21	24	22	27	30	30			
1 EDU		٤		94	Nameter Diameter Diameter	SOW FULL	4		20.91	23.90	25.96	27.60	28.84	29.56			
••		,	ł	9 <u>0</u>	meter U		(MCMCs)		16.73	19.12	┝	-	H	\vdash	┞		
	Dine Decion		4	- De	_		E		0.0050	1.0050	-	╁	╁	Ł	Ł		
ı	ľ	ł	┙	_	ength	_	Œ		2.500	F	T	-	Ŧ	۰	ť	200	
			_		=	evation	(¥				T	200		Ť	Ī	4	
			12	Upstream Downstream	Invert	Elevation	3		670	2/2	205	204	P.	238	230		
			7	Cumulative Ur	let Weather	Flow	(S)	82. ~	200	110	8,72	10.87	17.80	14.39	15.3/		
		Maximum Wet Weather	13	1		Flow				1	1	2.15	1.93	1.59	96.0	15.37	
District	ons	Maximum	15	15/8	- 2	Flow	(moo)		CTCT	_		┙		712	438	968'9	
Utility C	alculat		=	Take I	Weather	Home	(000)		2,181,465	1,765,665	1,686,960	1,389,960	1,250,370	1,024,650	631,125	9 030,195	
reen Valley Special Utility District Drainage Area D - 1 EDU	Pipe Flow Design Calculations	Jeather Flow	۶	No.	Cummanure	Dry wedged	100	220	359,905	651,210	929,530	1,158,850	1,365,140	1,534,190	1,638,315		
Green Vall Drain	Pipe Flo	Average Dry V		5	2	Weather		(600)	359,905	291,305	278,320	229,320	206.290	169.050	104,125	16.96 345	1,000,013
		-	1	•	100		į) (EDU) [1469	1189	136	936	5	8	425	ŀ	ž
		Bonnishon		7	Development Total	Density		(acres) (acres) (1 EDU/acre) (EDU)	1.0	-	-	-	-				
			Acc	9	Total	Area		(BQT68)	1460	1180	71.35	350	200	200	430	3	6687
			Contributing Area	2	Right					+	$^{+}$	+	+	8	+	-	2 4095
			ರ	*	H	n Side	on Area	(acres)	7	+	+	+	+	+	+	-	2592
			ocation	m	Down	-	Collection Collection	Point	_		7000					9	
			Sewer Main Location	7	S	Stream	Collectio	Dodne		Pipe DD Upstream	Pipe DD CP DD-1	Pipe DD CP DD-2	Pipe DD G DD-3	Pipe DD CP DD-4		8	E C
			Sex	-	8 6	2				Pipe DD	Pipe DD	00 ed	Pipe DD	Pe Di	P D	200	

0.013 80%
Manning's Roughness Coefficient = Percent of Pipe Flowing Full =
GPD/EDU GPD/EDU GPD/EDU
245 735 1485
Average Dry Weather Flow = Maximum Dry Weather Flow = Maximum Wet Weather Flow =
GPD capita/EDU EDU/acre GPD/capita gallon/acres served
245 3.5 1 70 3 750
Design Parameters: Residential Single Family Units (EDU) = Population per EDU = Development Average Density = Wastewater Demand = Maximum Flow Peak Factor = Inflow/Inflitration =

Manning's Roughness Coefficient = Percent of Pipe Howing Full =

GPD/EDU GPD/EDU GPD/EDU

245 735 1485

Average Dry Weather Row = Maximum Dry Weather Row = Maximum Wet Weather Row =

gallon/acres served

GPD capita/EDU EDU/acre GPD/capita

245 3.5 1 70 3 750

Residential Single Family Units (EDU) =
Population per EDU =
Development Average Density =
Wastewater Demand =
Maximum Flow Peak Factor =
Inflow/Infiltration =

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Ì			77	Pipe		Velocity			- 1	4.21	4.67	5.11	5.11	5.5	5.5	5.0	5.92	0	2	1	4	
;	EDO		17	S. C.			Ξ.			128	71	24	24	27	27	ç	Ç	S	3	8		
1	H		20	1-		Diameter Diamete	80% Full	(inches)		17.16	19.64	21.69	23.60	25.15	26.30	27.42	26 25	20.27	78.04	29.77		
		ē	61		Ľ.	Diameter Di	Actual			13.73	15.71	17.35	18.88	20.12	21 12	6	20 50	00'77	73.07	23.41		
		Pipe Design	×			Slope		E		0.0050	0.0050	0.0050	0.000	0000	2000	200	0.000	2000	0.0050	0.0050	0.0050	
			L	+		Length		3		2.500	۲	۰	۰	Ť	t	+	+	+	┪	3,400 (0 002'42	
			-		DOWNSTREAM	-	Elevation	£		55		Ī	666	200	550	070	615	610	902			
				2	Costream	Invert	Elevation	£		650	EAR	3	3	SS	630	625	620	615	610	605		
			1		Cumulative		Flow	SE	5	196	3,01	2.1	6,73	8.43	66.6	11.36	12.58	13.62	14.39	14.97		
		7.00	Maximum wet wedue	13	Wet	Weather	Hone	Û	8	200	8	2	1.56	1.70	1.56	1.37	1.21	2.	0.77	5	14.67	
	Jistinet U	Suo	Махимом	77	Wet	Wenther		(mun)		6/3	9				702	616	545	466	787	361	ľ	1
	F - 1 ED	Calculat		1	Wet	Menther			200	1,05/,733	1,072,170	1,009,800	1,009,800	1,097,415	1,011,285	886,545	784,080	671.220	408 050	306 366	3/3/03	2/0/4/19
	Green Valley Special Utility District Drainage Area F - 1 EDU	Pipe Flow Design Calculations	Average Dry Weather Flow	10		Cummacae	My wednes	MOL	77.3	207,515	384,405	551,005	717,605	898,660	1.065,505	1 211.770	1.341.130	1 451 870	1,731,000	1,534,130	1,590,175	
	Green Val	Pipe Fi	Average Dry	•		A	Weather	A C	(049)	207,515	176,890	166,600	166,600	181.055	166.845	146 265	120 360	110 740	04/'0TT	82,320	61,985	1,596,175
			Ę	•	°	0	2			847	722	989	680	2,0	189	200	520	9 5	455	336	253	6515
			Population	,	•	Development	Density		(1 EDU/acre)	1.0	1.0	-	2	2	2	Tio	0.	21	61	1.0	1.0	
			5		٥	Total	Area		(acres)	847	727	989	200	96	ŝ	8	à	228	425	336	223	6515
			Contribution Area		2	Right	Side	Area	(socies)	614	8	280	200	1	3	3	215	£	363	520	82	4088
			1	3	4	Let.	Side	Area	(socies)	1	i i	2 6	163	53	3	3	587	83	2	28	171	2427
				1500	m	Down	Stream	Collection	Point	i E	1 2		2 1	±	GH.S	9 # 6	9#7	ტ #	9 H 9	OFF 10	G F 11	
				Sewer Main Location	~	9	Stream	F	Point	mon to	+	+	7 # 5	д В	₽ ₩	QFF S	9 ₩	OFF7	Q FF 8	╁	╁	Total
				New York	,	Pipe		ا			1			- !	P e F	Pipe FF	Pipe FF	Pipe H	Pine FF	9	k	1

Manning's Roughness Coefficient = Percent of Pipe Flowing Full =
GPD/EDU GPD/EDU GPD/EDU
245 735 1485
Average Dry Weather Flow = Maximum Dry Weather Flow = Maximum Wet Weather Flow =
GPD capita/EDU EDU/acre GPD/capita gallon/acres served
245 3.5 1 70 3 750
Pesian Parameters: Residential Single Family Units (EDU) = Population per EDU = Development Average Density = Wastewater Demand = Maximum Flow Peak Factor = Inflow/Inflitration =

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	1	U)			22		2	Velocity		Actual	(3 (3 (3)			3.73	4.21	4.67	4.67	5.11	5 52	9 2		6.31
		Ē				21		2	Diameter		Mominal	(inches)			CT	18	21	21	24	27	5	3	2
			j			2		<u>R</u>	Diameter Diameter		100.00 100.00	(inches)		** 5:	Į.	16.50	19.13	21.26	23.43	26.15	28.36	20.00	26.8/
					5	61	Dina	2	ameter	-	_	(Inches) (32.04	0,70	13.20	15,30	17.01	18.74	20.92	22.69	2	74.70
				Dine Beer	In The Design	18	o io	<u> </u>	Slope			(H)		ONED	+	-	4	0.0050	0.0050	0.0050	╁	ł	OCON.O
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						10	Jostpeam Downstream		Inver	Flourthun				630	1		209				263		
					•	2	Upstream		TINGLE	Floration	•		-	25	95	670	618	200	296	585	573	563	-00
				Į.	ı	\$	Cumulative	_	wer weamer		•	ı	0.83	1.88	2,75	3,23	4.81	5.5	8.27	11.09	13.76	17.76	27.15
				Maximum Wet Weather		,	¥et ×	-	1955	FIOW.	3		0.83	1.05	4.37		67.	1.5/	1.89	2.81	2,67	3.50	
1177	NSTACT.	_	ons	Maximum	5		¥et	Manthen		HOW	(man)		3/4	69	615		707	5	È	1263	1199	1571	
114:11:4.		G - 1 ED	Calculati		=		¥et	Westher		¥0E	(GPD)	230 002	237,033	675,675	RREDEO	1 011 305	1 015 740	1,015,010	1,219,185	1,819,125	1,727,055	2,261,655	
lev Specin	green valley special cullity district	Drainage Area G - 1 EDU	Pipe Flow Design Calculations	Veather How	٤		Cumulative	Dry Weather		Flow	(GPD)	90 035	00,333	200,410	346.430	C12 77E	220,052	000,000	002,000	1,184,125	1,467,060	1,840,195	
Green Va	פוענו אם	Orai	Pipe Fl	Average Dry Weather Flow	•		5	Waather	1	¥0¥	(Q45)	20 035	00,333	111,475	146.020	166 845	167 580	201 100	207,102	300,123	284,935	373,135	
				5	-			2		_		3,63	3	455	296	189	289	3 5	170	Ş	1103	1523	
				Population		Detroloment	Development 100	Density			(EDU/acre)	٤		1.0	1,0	6.	-			2 .	0.1	1.0	
				Area	9	Total	9	Area			(acres)	363		450	236	681	89	821	1	277	1103	1523	7644
				Contributing Area	2	olohe		Side		2	(acres)	767)	1	129	8	160	207	3 2	26.	1410	2227
				Ŝ	4	4		Side		2	(acres)	96	Ļ	4	452	552	쟔	١.,	L	+	+	4	72.17
				Cation	m	Down	3	Steam	Collection	275	Point	CP GG-1	200	3.55	969-3	Q 66-4	Q 66-5			200			
				Sewer Main Location	7	٤	3	Stream	Collection		Point	Mpe GG Upstream			CP 6G-2	CP GG-3	9 664	Pine GG CP GG-5	رو روجو				Total
			ļ	še.	1	Pine		2				Pipe 6G	000	R	Pipe GG	Pipe 6G	Pipe GG	Pipe GG	Dine GG		000	200	

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GPD/EDU G GPD/EDU SS GPD/EDU
245 735 1485
Average Dry Weather Flow = Maximum Dry Weather Flow = Maximum Wet Weather Flow =
GPD capita/EDU EDU/acre GPD/capita galion/acres served
245 3.5 1 70 3 750
Residential Single Family Units (EDU) = Population per EDU = Development Average Density = Wastewater Demand = Maximum Flow Peak Factor = Inflow/Infiltration =

Manning's Roughness Coefficient = Percent of Pipe Flowing Full =

	(Green \			Utility I	District		
			3 E	:DU/a	cre			
		Pip	e Flow	Design	Summ	ary		
Pipe	Basin A	Basin B	Basin C	Basin D	Basin E	Basin F	Basin G	Total
Diameter	Pipe	Pipe	Pipe	Pipe	Pipe	Pipe	Pipe	Pipe
	Length	Length	Length	Length	Length	Length	Length	Length
(in)	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)
8		, ,						0
10								0
12								0
15	2,600							2,600
18	11,200	2,600	2,600				2,700	19,100
21	13,800	2,800	2,800				2,700	22,100
24	9,500	2,800	2,800		4,600	2,500	2,500	24,700
27	5,500	2,600	2,800	2,500	7,350	2,500	2,500	25,750
30	3,500	9,400	5,400		12,600	5,000	2,500	38,400
33			7,600	2,500	13,000	2,500	2,800	28,400
36	5,000			5,300	14,450	7,700	2,900	35,350
42	11,400			5,600	31,950	7,000	3,000	58,950
48					5,800			5,800
54	+	-						0
60	+				7,000			7,000
66					5,000			5,000
72	 				23,100			23,100
Total	62,500	20,200	24,000	15,900	124,850	27,200	21,600	296,250

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Utility Dis - 3 EDU alculation	*		==	4 - 500	
Sreen Valley Special Utility District Drainage Area A - 3 EDU Pipe Flow Design Calculations	Veather Flow		10	Part Coursellands	
Green Vall Drain Pipe Flo	Average Dry V	,	.		5
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	Populat		,	Description	Personal Personal
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		Ę	8	-		(mcnes)	œ	2	7	24	1	T		24	22	22	30				9	2 2	1 7	†	T		18	18	7	1	\dagger		36	36	1	\dagger		42	1	+		42			
		20	8	_	80% Full	+	18.77	2	200	24.18		T		23.26	26.06	27.05	27.67			20.5	RE	20.00	20.74		t		15.60	18.03	20.34	1	+		34.62	35.57	\dagger			38.90	+	+		42.66	H		
	ab	19	9 <u>7</u>	7	Actual	+	15.02	16.43	17,63	19.34		-		18.61	H	21.64	-			10	7.30	16.08	+	╁	t	-	Н	14.42	+	1		-	27.69	H	\dagger		Н	31.12	1	+		34.13	Н	+	
	Pipe Design	18	1	Slope	•	t	0.0060	╀	Ļ	L	0.0058	l		-	\vdash	0.0050	4	0.0050	1	0.000	+	4	0.0080	1	\mid	Н	-	4	4	0.0050			0.0050	Н	200	-	Н	4	0.0050	+	Н	H	0.0050		
		17	8	Length	•	1	t	۰	2,500	Т	1	_				2,500		12,500	1	+	+	2,800	1	-		H	3,000	+	-	17,000 0.	+			2,500 0.	+		H	7	4,500	\dagger	-	Н	4,900 0.	+	
		16	Ξ	Invert	Elevation (m)	1	T	Ī	555		Ī					553				Ť	İ	545	Ī	Γ		Ħ	1	536	T	1			535	7			1	525	1		П	510	4		
		15	Ε	Invert	Elevation (F)		99	585	570	555				290	278	565	SSS			003	185	563	545				220	243	220	1			240	235	1			530	1			525		+	
	. 1	14	Cumulative	Wet Weather	8 €	3.55	5.02	6.38	7.70	8.99			4.58	8.12	10.99	12.13	17.03		7	302	4 65	6.23	7.56			1.04	2.79	4.11			8,99	21.89	23.42	25.18		29'5	30.85	7; 8	1	7.56	39.52	40.89		40.89	
	Maximum Wet Westner	13	_	5	€ €	3.55	1.47	1.36	1.32	1.29	8.99		4.58	3.53	2.87	1.14	2	17.69	2	1.47	162	1.59	1.33	7.56		2	1.75	1.32	2 67	ì	8.99	12.89	7.	1.76		2.67	25.18	11.1	21.30	7.56	31.96	1.37	40.89	40.89	
S	axmadm m	12	_	Weather	(Wab	1592	629	612	593	581	4,036		2056	1586	1289	513	1	/8//2	ş	199	728	712	595	3,392		88	8	160	2 EAE	27.27	╁	Н	069	30,1	╀	H	_	200	4	╁	14344	4	18,352	18352	+
alculation	1	11		Weather	(GPO)	2,293,080	948,555	065'088	853,995	836,265	5,812,485		2,960,910	2,284,215	1,855,740	/38,/50	ŧ		1 001 745	951,510	1.049.025	1,025,385	_	4,884,615		673,740	1,131,765	1.007.655	1	4	5,812,485	8,333,100	992,880	1,134,720		L	2	70,000	_1_	┸	0	-4	26,426,565 1	26,426,565	1
Ipe Flow Design Calculations	MOLL ISING	21	Cumulative	Dry Weather	(Q.15)	570,360	806,295	1,025,325	1,237,740	1,445,745						2 072 700	T	Ī	249 165	485,835	746,760	1,001,805				167,580	449,085	911,400	t		П		+	1		Н	4,959,045	T		т	6,352,605	7	7	6,573,105	Ť
Pipe Flov	Avelage My w	6	Ā	Weather	(GMD)	570,360	235,935	219,030	212,415	208,005	1,445,745		736,470	568,155	461,580	122,730	200 120 1	20,77,00	249.165	236,670	260,925	255,045	213,150	1,214,955		167,580	201,505	250,635	911.400		1,445,745	2,072,700	246,960	4.047.645		911,400	170 605	1/0/00 5.127 KED	2000	14,955	5,137,650	0,500	,573,105	6,573,105	ŀ
Ţ	1	∞	Total	3	(EDU)	2328	963	\$	867	-	5901	-	+	-	+	8 6	٠	+	۲	✝	Н	1041	-	4959	\dashv	+	+	1023	╀	╀	Н	8460	+	+	₩	\vdash	16521	+	+-	↤	20970	4	-	26829	
Domination	nondo .	,	Development	Density	(3 EDU/acre)	3.0	3.0	3.0	3.0	3.0						300	Ī		3.0	3.0	3.0	3.0	3.0			3.0	200	3.0			3.0	3.0	3.0				3.0	Ī			3.0			3.0	
Area		7		ATG	ŝ	Н	321	85	88	283	1967		201	E 6	870	167	2830		339	322	355	347	8	1653	000	877	288	34	1246		1967	2820	85	5507		1240	243	0669	T	1653	0669	25.5	2543	8943	360
Contribution Area		^	Roht L	Area	(acres)	220	1	129	114	۲	88	,	7	787	¥ 5	133	700		246	195	762	220	240	1163	;	3	3,5	22	194		986	ğ	103	1.	П	-+	2022	٠	t	1163	2461	+	1	\neg	21
3	,	•	¥ ;	Area	ĭ		171	1	175		282			_L		2 2	Г		L			127	_	8 8	į	217	252	692	1046					3302	ı	1	3302	1	1	11	4529	+	+	5227	Г
rion	,	2	Down	·	Point	CP AA1-1	O A41-2	Q A41-3	P WIT	C WI-5			- K	G AA2-2	200	Q A41-5			CP AA3-1	CP AA3-2	CP AA3-3	C AA34	C AA6-1			3 444.5	CP AA4-3	CP AAS-2			CP AA1-5	O AM1-5	36.5	2000		C MAS-2	9 A85.2			_	Q A86-1		Ī	CP AA7-1	2 AA8-1
Sewer Main Location	,	1	do E	Collection	Point	Upstream	CP AA1-1	CP A41-2	Q AA1-3	C ANI-4	ig of	_	Upstream Co. 1	G 482-1	2.747.5	Q 424	1257					CP A43-3						CP AA4-3		П				Total			Q 441-5				O AAS-2			CP AA6-1	
Sewei	•	4	<u> </u>			Pipe AA1	¥.	Pipe AA1	Pipe AA1	Ž.	_	-	Pipe AA2	AA2	242	Pipe AA2		l	Pipe AA3	Pipe AA3	AA3	Pipe AA3	Ş	1		Dine AAA	4	Pipe AA4	4	\vdash	Pipe AA1	3	AAR		_	Pipe AA4	Ploe AA6	٥			Pipe AA6		\mid	Pipe AA7	AMA

Design Parameters:

Manning's Roughness Coefficient = Percent of Pipe Flowing Full =
GPD/EDU GPD/EDU GPD/EDU
245 735 985
Average Dry Weather Flow = Maximum Dry Weather Flow = Maximum Wet Weather Flow =
GPD capita/EDU EDU/acre GPD/capita gallon/acres served
245 3.5 70 3 750
Residential Single Family Units (EDU) = Population per EDU = Development Average Density = Wastewater Demand = Maximum Flow Peak Factor = Inflow/Infiltration =

m		22		<u>2</u>	Velocity	Nominal	(m/ear)			5.23	5.58	6.10	20.0	0.00	6.92	49	7.49		
S		21	ł		_	=				81	21	24	ł	1	1	30	30		
3 EDU		20	╁	7 Pe	Diameter Diameter	80% Full No		T COLUMN		18.10	20.97	22 54		25.34	27.55	28.25	30.67	ŀ	
(')				2	meter Dia	Actual 800		Inches) III		14,48	16.77	+	+	-	22.04	22.60 2	24.53		
	Pipe Design		١	8	Slope Diar	4		(T)		0.0077 14	160071	+	1	0.0077 20	0.0068 22	0.0080 22	0.0080 24	L	
i	٦		4	_	S thous	_	_	(E)		2,600 0.0	F	٠	+	2,600 0.0	4,400 0.0	2.500 0.0	۰	Т.	
l:		ļ	10	u	_			-	-	580	T	T					450	Ī	
		-		Instream Downstream	1				-	Ü	1				-	-	-	1	
			2	F	_	T TOTAL		€		89	3	8	265	3	520	400	1		-
			3	Cumulative		Wet westing	₩ 6	(SE)	S	200	07.0	33	10.01	43.5	14.88	17.34	77.77	1777	
		Maximum wer wearner	2	10,00		b	₹ E	(ggs)	Ę	7.30	770	2.19	7,66	3.63	200	257	253	1	21.44
istrict _	2	Maximum	12			Weather	HOM	(man)		1001	1014	985	198	1170	970,	3 5	105/	- 1	9,624
Utility D 8 - 3 EDU	Calculad		••		Mer	Weather	HOL	(CBD)		1,8/3,4/0	1,459,770	1,418,400	1 719 810	02 0 30 7 F	1,090,17	1,450,905	1,521,825	2,718,600	13,858,950
een Valley Special Utility District Drainage Area B - 3 EDU	Pipe Flow Design Carculations	feather flow	6.	3	Cumumanave	Dry Weather	H		a.	465,990	829,080	1,181,880	1 600 650	1,003,000	2,031,540	2,392,425	2,770,950	3,447,150	
Green Vall Drair	Pipe Flo	Average Dry Weather Flow		,	2	Weather			(CL)	465,990	363,090	352.800	027 550	47//76	421,890	360,885	378,525	676,200	3,447,150
		r	ļ	ř	Total	EDG	}		-1	1902	1482	1440		9	1722	1473	1545	2760	14070
		Population		7	Development	Deneilly			(acres) (3 EDU/acre)	3.0	3.0	20		3.0	3.0	3.0	3.0	3.0	
		Ares		9	Total	_	B		_	634	404	460	2	282	574	491	515	920	4690
		Canadan Area			Heia A	-	250	Area	s) (acres)	367	H	5	+	216	190	285	╁	╁	H
		K	3	•	5	1		ion Area	t (acres	(<u>)</u>	+	+	4	4 8	-5 384	Ļ	╀	╀	H
			Location	m	Posterio			Collection Collection	Point	70 BB.	+	+	C 88-3	9887	CP 88-5	╀	+	+	11
			NEWS MAIN LOCADON	2	1	3		Collectic	Point	F	200	5	2 da 2	R CP 88-3					
			አ	-			8				2	000	Proc 55	Pine 88	Dine R.R.				R E

0.013 80%
Manning's Roughness Coefficient = Percent of Pipe Flowing Full =
GPD/EDU GPD/EDU GPD/EDU
245 735 985
Average Dry Weather Flow = Maximum Dry Weather Flow = Maximum Wet Weather Flow =
GPD capita/EDU EDU/acre GPD/capita 0 gallon/acres served
245 3.5 3 70 750
Design Parameters: Residential Single Family Units (EDU) = Population per EDU = Development Average Density = Wastewater Demand = Maximum Flow Peak Factor = Inflow/Inflitration =

	C)			77	P De	Valority		-	(11/8eC)		5,23	20.0	6.10	6.60	100	7.75	2 2	7.83	3
	ECH C)		ļ	77	200	Diameter Diameter			(anches)		27	21	24	27	į	3	3	3	7
	(/)		5	1	<u>P</u>	Diameter	7000		(Inches)		17.32	20.77	23.46	25,50	27 02	20 74	21.25	37.40	32.45
			Sign	9	4	2	Diameter		Actual (notes)	IIICII ES I		13.85	16.61	18.77	20.47	22.33	22 70	25.00	25.00	76.67
			Pipe Design	9	9	<u>2</u>	Slope	1	٤			0.00	0.0071	0.0071	0.0071	0.0071	0.0077	0.000	2000	200
				٤		2	Length		(1	2000	2,000	2,800	2,800	2,800	2,800	2,600	2,600	2,000	55.7
				16	,	_	Invert	Flavation			400	200	260	540	220	200	480	460	440	F
				15		протодо	Invert	Flencation	•		903	3	280	095	540	520	200	480	460	3
			ž	14		Cumuladve	Wet Weather	H A	Ę	15,	4 50	4.30	7,16	9.92	12.50	15.77	19,37	22.79	24.84	-
			Maximum Wet Weather	13	17.00	Ď	Weather	Flore	ŧ	212	2 46	2	2.58	2.75	2,58	3.27	3.60	2.92	2.55	
istrict	_	Suc	Maximum	12	1000		Weather	Flow	(mon)	957	118	5	1159	1235	1159	1469	1615	1311	1143	
	C-3 EDU	Calculation		11	40/80	Į.	Weather	Flow	(GPD)	1.371.120	1 580 700	2000	1,669,575	1,778,910	1,669,575	2,115,780	2,325,585	1,888,245	1.645,935	
een Valley Special Utility District	Drainage Area C - 3 EDU	Pipe Flow Design Calculations	rage Dry Weather Flow	97	Comments		Dry Weather	Flow	(GPD)	341.040	736.470	2000	1,151,/45	1,594,215	2,009,490	2,535,750	3,114,195	3,583,860	3,993,255	
Green Val	Drai	Pipe Fl	Average Dry	6	2		Weather	Flow	(GB D)	341.040	395.430	100	415,275	442,470	415,275	526,260	578,445	469,665	409,395	
		İ	tion	∞	Total				(EDG)	1392	1614	1	Céa	1806	1695	2148	2361	1917	1671	
			Population	7	Develorment		Density		(EDU/acre)	3.0	3.0		0'0	3.0	3.0	3.0	3.0	3.0	3.0	Ş
			7 Area	9	Total	_	Area		(acres)	\$	538	2	S S	200	265	716	787	639	557	200
			Contributing Area	s	Right		_	Area	s) (acres)	314	365	5	2/2	331	553	529	203	119	152	
			8	4	Left	_		oni Area	(acres)	1 150	2 173	163	4	+	+	+	4	3 520	_	
			ocation	6	Down	C	ineans (Collection Collection	Point	CP CC-1	3 5 5	200	3 8	3 8	3	+	+	ဗီ ဗီ		
			Sewer Main Location	7	9	200	E ST COL	Collection	Point	Upstream	ე	500		38	1	200		9 6-7-	ipe CC CP CC-8	COUNTY OF STREET
		_	8	-1	Ppe	٤.	}	_		Dipe CC	Pipe CC	Dine CC	2	2 2	2 2	200	De CC	De CC	Pe CC	1

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	3			7	4	Velocit	_	_	¥ 2		57.7		6.31	6.69	69'9			7.41			
	2			23	Pipe	Diameter Diameter		Nomina	(inches)		7.6		2	36	36	•	76	42			
γ) T			20	Pipe	Dismeter		80% Full	(inches)		27.67	70.77	30.93	33.60	35.73		37.33	38.26			
			E E	19	Pipe	-		_	(inches)		1000	77.00	24.75	26.88	28 58		29.86	30.60			
			Mpe Design	\$	Pipe		_		£		0000	0.0050	0,0050	0.0050	OVE	2	0.0050	0.0050	0.0050		
				17	Pipe				E	ŀ	t	2,500	2.500	t	۲	2,000	3,000	2,600	L		
				16	2		INVEST	Elevation	£			262	27.	248		930	230	225			
				15	į			Elevation	E			220	CGS	200	5	£	538	925			
			8	2	T	Campagaga	Wet Weather	Alcon.	3	T	2/.0	17.15	36 61	21.53	51.03	25.48	28.63	20 50	30,30		
			Maximum Wet Weather	;			Westber		{		2/5	44		5.19	4.28	3.85	2 15	25.	5	20.00	
Strict Strict		38	a virmitth		1	ě	Washer				3012	OVVC	2	2331	1921	1728	2446	OTL	8/2	13,722	
	3 EDU	Calculatio		١	=	ĕ					4.340.895	207	3,513,433	3,356,880	2,765,880	2 488 110	200000	7,038,930	1,255,875	119.760.085 13,722	
Green Valley Special Utility District	Drainage Area D - 3 EDU	Pipe Flow Design Calculations	The second second	WGBUIGH LIOW	10	Cumulative		Dry weather	HOW	(GPD)	1 079 715	20,000	1,953,030	2,788,590	3,476,550	A ABC 420	7,030,7	4,602,570	4,914,945		
Green Val	Drain	Pine Fic		Average Dry	6	200	1	Weather	¥o£	(epo)	1 670 715	1,0/2,/12	873,915	834,960	687.960	000	018,8/0	507,150	312,375	370 710 7	ALC: TELL
				ē	80	Total L		2				Ì	3567	3408	280R		2526	2070	1275	20061	70007
				Population	7	Paramatan day	Treatment of the second	Density		(acres) (3 EDU/acre)		5.0	3.0	3.0	3.0	3	3.0	3.0	3.0		
				Area	9			Area				1469	1189	1136	200	3	3	069	425		6687
				Contributing Area	8		KIGHT	Side	Area	•		1068	215	7,5		OTO	466	283	128		2003
				S			5	Side	Area	,		491	┸	1	4	320	376	407	6		2592
				cation		,		Stream	·	Delant		CP DD-1	200			5	C DD-5	1_			İ
				Sewer Main Location	,	•	9	Sheam	Coffertion		LIDE	Inchream	200		200	Š	460	200	3 8	5	Total
				3		7	Pine	ę	}	_		Director Inches			Pro DO	Pice 00	200			20	

Manning's Roughness Coefficient = Percent of Pipe Flowing Full =
GPD/EDU GPD/EDU GPD/EDU
245 735 985
Average Dry Weather Flow = Maximum Dry Weather Flow = Maximum Wet Weather Flow =
GPD capita/EDU EDU/acre GPD/capita gallon/acres served
245 3.5 3 70 750
Pesion Falamenes: Residential Single Family Units (EDU) = Population per EDU = Development Average Density = Wastewater Demand = Maximum Flow Peak Factor = Inflow/Inflitration =

Manning's Roughness Coefficient = Percent of Pipe Rowing Full =

GPD/EDU GPD/EDU GPD/EDU

245 735 885

Average Dry Weather Flow = Maximum Dry Weather Flow = Maximum Wet Weather Flow =

gallon/acres served

GPD capita/EDU EDU/acre GPD/capita

245 3.5 3 70 70 750

Residential Single Family Units (EDU) =
Population per EDU =
Development Average Density =
Development Average Density =
Wastewater Denmand =
Maximum Flow Peak Factor =
Inflow/Infiltration =

ш		1	Pipe	Velocity	(F/8ec)	3	9.9	9 4	7.42	7.42	7.86	8.09	8.96	96.50	8.96	29.	Ī		5,92	6.31	6	0.0 14	7.41	7.41	7.41	7.41	14.	8.10	0. 0.		. 1.	5.52	5.92	6.69	1		9.40	9.40	9.40			20.02	10.62	0.62	0.62	0.62	0.62	0.62
EDO		ļ		Nominal	(Inches)	;	75	2 2	33	33	36	36	7	2	42	;	t		30	2	8 3	8 3	t		42	\dagger	t	Н	+		†	h	8	#	t		\dagger	9	t		Н	†	27	Н	+	+	Н	+
3 三		Т	_	80% Full		12.20	75.57	30.44	32.61	33,45	35,39	36.80	38.14	40,30	41.07	47,123	t		30.76	33.09	25.00	38.18	39.21	40.02	40.77	41.48	42.65	43,35	43.35		24.40	26,55	29.58	34.42	t		57,83	58.39	50.60			15.53	80.89	58.79	9.80	1.06	71.49	1.63
			_		(Inches)	20.05	23.53	24.36	26.09	26.76	28.31	29.44	30,51	32.24	32.85	23.00	T	Н	+	+	+	t	Н	Н	32.62	+	╁	34,68	╁		$^{+}$	Н	23.66	╫	\dagger		╁	46.71	╫		Н	╁	54.46	Н	+	Н	Н	+
	Ofne Declar	K e	Plpe		E	4	4	┺-	1_	Ш			\perp		0.0073			Н	-	4	+	0.0050	L	Ш	_	4-	+	Н	0.0050		+	Н	4-	0.0050	P. C.		┷	0.0050		-	ш		0.0050		_[_		L1	
			Ple		g										2,600				_	-	-	_		_	2,500	-	2,800	П	34,750					4,000			1	2,500				_	3,000		_	j		_
		٤	ownstream	Elevation	ε	118	ê	733	754	735	716	697	629	640	621				269	27.3	999	98	652	3	989	620	612	\$	900		846	627	808	570			590	580	2			Τ	556	Ī	Ī			T
		=======================================	,	Elevation	E	830	811	792	773	754	735	61,0	829	629	3 2			9	90	684	9/9	88	099	3	2 2	879	620	275	56	l	599	5 46	208	589			8	590	000		200	2,66	561	356	246	541	236	152
		14	Cumulative Wet Weather	_	T	12.27	15,88	19.55	23.48	25.13	29.20	25.55	39.70	42.46	44.65			13.13	17.05	24.17	27.33	30,41	32,64	34.40	35.24	39.52	40.85	42.68	27.00	25.5	9.22	11.55	19.15	23.06		46,39	92.05	94.42		23.06	120.26	24.95	142.21	146.17	156.29	159.41	161.99	107.80
	Vet Weath	13	Westher	Flow	E	3.60	3.61	3.67	3,93	59:	80:	100	Š	2.77	1.74	46,39		13.13	5 5	9	3.16	3.07	2.24	1.85	51.5	1.57	1.33	1.83	42.68	2.57	3.65	2.33	3,75	3.91		46.39	2.97	2.38	97.20	23,06	97.20	7,68	7.26	3,97	4.31	3.12	2.58	62.8K
2	Maximum Wet Weathe	12	West	₩	(Mos	1615	1619	1646	1765	82	1830	1490	1363	1242	280	20,820		2895	1654	1525	1420	1379	1003	628	767	902	297	823	19,156	2499	Н	+	+	H	Н	20820	┨┪	-	1 1		3144							
- 3 EDU	I	11	Weather	Flow	5,605,635	2,325,585	2,331,495	2,369,910	2,541,300	1,063,800	2,674,775	2,145,330	1,962,120	1,787,775	1,415,445	29,981,430		8,483,805	2.381.730	2,195,565	2,044,860	1,985,760	1,444,995	1,193,820	1,105,170	1,016,520	859,905	1,184,955	27,584,925	3,599,190	2,358,090	1,504,095	2,423,100	2,529,480 14,905,020		27.584.925	1,920,750	1,536,600	1-1		4.577.060							
Drainage Area E - 3 EDU Pipe Flow Design Calculations	Average Dry Weather Flow	10	Cumulative Dry Weather	Flow	1 394 795	1,972,740	2,552,655	3,142,125	3,774,225	4,038,825	2 350 620	5.893,230	6,381,270	Т	Т	П	Ħ	+	+	۲	-	Н	+	+	6,099,765	Н	H	+	H	\neg	П	т	3,078,180	Т	П	14,318,535	H	-	H		20.457.255		_	_	1	-	_	+
Drai Pipe Fik	Average Dry	9	Dry Weather	Flow	1.394.295	578,445	579,915	589,470	632,100	000,000	665 175	533,610	488,040	444,675	279,300	7,457,310	2010	636 510	592,410	546,105	208'620	493,920	359,415	280,740	274,890	252,840	213,885	CC/120	6,861,225	895,230	586,530	3/4,115 610 605	602,700	3,707,340		6,861,225	477,750	445,410	15,623,895	3,707,340	+-	Н	+	+	+-	+	╁	╀
	ion		Total EDU	(EDS)	5691	2361	2367	5406	2280	2676	27.15	2178	1992	1815	1140	30438	1	25.00	2418	2229	5076	2016	è	145	1122	1032	873	200	28008	3654	238	75.20	2460	2568	Н	28005	₩	-	н	15132	_	Н	4764	+	+	-	+	+
	Population		Development Density	/3 Ehll/acms	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0			3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	0.0	3.0	3.0		3.0	П			3.0	3.0							ľ
	Area	9	Area	(action)	1897	787	789	805	9 S	e e	506	726	\$	şę	380	10146	2071	998	908	743	69	2/9	ę Ş	383	374	¥.	5	0	9335	1218	86	£ 2	820	5044	,,,,,,,	9335	650	909	21257	45	1532	1680	1588	1221	942	282	161	35619
	Contributing Area	2	Sight Side	Area	1455	476	424	£	<u> </u>	267	52	255	248	15,24	130	4933	1000	519	484	\$	419	908	2012	135	142	191	121		5849	20	£ s	209	879	2203	1	5849			12240	2203	607	3	S C	621	267	17 665 682	5	18954
	ර	7		Area	_		_1		_	1_	1	1 . 1		- 1		2313									232									2841		3486			9017	2841					375	2 0		16665
	cation	_	Stream	Collection	CP EE1-1	O EE1-	G EE		3 6	O FE1-7	C EEF	CP EE1-5	CP EE1-1	3 6	O EE1-13		CB EE2.4	O EE2-2	CP EE2-3	CP EE2-4	O EE2-5	G EE2-6	9 65	Q-EE2-9	O EE2-10	O EE2-11	O EE2-17	O EE1-13		CP EE3-1	0	Q EE	CP EE3-5		200	O EE1-13	96641	O EE4-3		G EE4-3	O EES-1	O EE5-2	9 6	CP EES-5	OP EES-6	D FES. 8	C EES-9	
	Sewer Main Location	~	Ε	Collection	Upstream	CP EE1-1	CP EE1-2	GEEL-3	1 1	Q EE1-6	CP EE1-7	Ø EE1-8	Q EE1-9	3 11.10	Pipe EE1 CP EE1-12 (Total	Inchesm	O EE2-1	CP EE2-2	Q EE2-3	O EE24	G EE2-5	G FE)-7	CP EE2-8	Pipe EE2 CP EE2-9 C	Q EE2-10	CP EE2-11	Ø EE2-13	Total	Upstream	966	9 60-3	CP EE3-4	1 2	landari I	Pipe EE2 Upstream (O EE1-13	CP EE4-2	Total	Pipe EE3 Upstream	CP EE4-3	9 65	Q EE5-3	CP EES-4	CP EES-5	Q E5-7	CP EE5-8	Total
	Sew	-	<u>₹</u> 8		'lpe EE1	Pe EE1	De EE1	130	P EF	100 EE1	he EE1	Pe EE1	Pe EEL		Po EE1		ine FF3	70 EE2	pe EE2	the EE2	20 EE2	Pe CEZ	De FEZ	De EE2	the EE2	De EE2	20 EEZ	Pe EE2		pe EE3	1	1	Pipe EE3	2	100	20	Pe EE4			De EE3	Pe EES	2 EES	20 665	se EES	Pe EES	S EES	De EES	

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L	L		22		R.	Velocity		(R/8eC		5.11	5 5	5	76.0	2.37	6.31	6.69	69.9	69.9	741	;	•			
	2		21		2	Diameter	Nominal	(inches)		74	33		200	2	33	36	36	36	5		7			
	3 500		20		<u> </u>	Teter D		(inches) (i		21	24.73	21.	78.0	30.54	32.56	34.17	35.49	36 56	37.33	3	2, .88	1		
•	7)		l	1	E		=	_	_	22.21	1	+		-	-	-		+	+	+	1			
		Lesign	٤		2	Diameter	Actual	(inches)		17.72	1	\$	22.46	24.43	26.04	27.33	28.30	30 25	79.67	8	30.31			
		Pipe Design	:	1	<u>P</u>	Slope		£		0.0050		0.0050	0.0050	0.0050	0.0050	0.0050	0.000	0000	2000	0.000	0.0050	0.0050		
			ŀ	77	Pipe	Length		£	l	2500	3	2,500	2,50	2,500	2500	250	200	200	3/,7	3,600	3,400	27,200		
			ļ	10	theam	_		P		245	2	640	2	630	425	200	200	2	910	605	909	ĺ		
			ľ	-	Downstream			£	L	1	5	æ	8	9		1		6	٥	ŏ	36	L		
				2	Upstream	Invert	Flewartion	9		945	200	2	2	25	003	3	200	020	615	910	8			
		t	†		=	_	_					8	٦			,	1	~	0	~	6	T	1	
			.	7	Cumudative	West Weather	i	3		200	7.	10.28	13.30	16.77	100	25.0	77.91	25.03	27.10	28.63	20.70			
			Wead	-		Masther		1		3.8/	330	3.11	2 11		8	3.11	273	2.41	2.07	2	1 16	9, 9		
٤			Maximum Wet Weamer	4.5		and the same		Ě		17.38	1482	1305	1305	200	1210	1397	1225	1084	928	690	210	l	4	
ig the	3:	acions	Max	ŀ	†		_		+	4		1	+	1	4	4	_		L		+			
Care Valley Special Utility District	Drainage Area F - 3 EDU	Pipe Flow Design Calculations		÷		10		10M		2,502,885	2 133 510	2 000 400	4,000,4	2,009,400	2,183,/45	2,012,355	1,764,135	1,560,240	1,335,660	000 880	375.00	CTO//L/	19,251,625	
Tr.	Areal	Sign (Flow	Ş			506	How	5	2 2	152 215		570,550,	2,152,815	2,695,980	3,196,515	3,635,310	4.023.390	355 610	0000000	2	4,768,525]	
	inage	OW DO	erage Dry Weather Flow	<u> </u>		Cumunadae	Ē	Ť,	9	622,545	1 157	1	6	7,15,	2,69	3,19	3,63	4.02	4 35		Š	•		
3		ipe Fi	age Dry			Ĕ	ather	How	90	545	0630	070/050	200	99,800	543,165	500,535	138.795	288 0.80	22,20	275	10,500	185,955	788,525	ĺ
K	5	Δ.	Aver			_	<u>\$</u>	_	<u>و</u>	729	Č	Ž į	\$	\$	35	S	43	25	3 66	3	7		4,	
			٤	. 1		Total	2			754		8	2	<u>\$</u>	2217	2043	2	1584	200	200	8	73	19545	
			Powellation		7	Developmen	Density		(3 EDU/acre)	٥		3.0	3.0	3.0	3.0	3.0	200	2 6	2	3.0	30	3.0		
			ŀ	-		Ш	_		=	ļ				L		-	+	+	+				L	
۱					•	Tota	Age		(acres	ł	Š	722	8	88	730	5	3 8		200	452	38	253	6515	
				Conditionand Area	2	200	Side	¥	(acres	ŀ	5	2	389	451	S	Ş	200	2 2	ŝ	363	250	82	4068	
				3	4	3	P.	V	(action)		5	343	291	2	230	+	9	107	ES	20	88		2427	
				500	m	Down	Cream	Collection	Dodent		1	OFF 2	OFF3	3 H 4				t	OFF8	0 H 0	OF FE 10	OF # 11		
				Sewer Main Location	-	 -	. [. 8	_	+	E	L	-	+		-	+	۵	_	0 H 8		┿	1	
				E STATE		-	, {		3 4	_		110	H	1	E	ŧ	E	Ŀ	FF 00 FF	H	E	: 1		
	L			_	ľ	1	•	=			Pipe F	80	1			Z	8	8	Pipe FF	ě			!	

Manning's Roughness Coefficient = Percent of Pipe Flowing Full =
GPD/EDU GPD/EDU GPD/EDU
245 735 985
Average Dry Weather Flow = Maximum Dry Weather Flow = Maximum Wet Weather Flow =
GPD capita/EDU EDU/acre GPD/capita gallon/acres served
245 3.5 3 70 3 750
Design Parameters: Residential Single Family Units (EDU) = Population per EDU = Development Average Density = Wastewater Demand = Maximum Flow Peak Factor = Inflow/Infiltration =

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	U		5	7	200	No.			(R/sec		7	77.	4.6/	5,11	55	5	26.6	6.31	6.69	2.7.5
	2		;	1	2	Diameter			(inches)		0	9	77	24	77	i	200	33	98	
	3 ED(20	3	9	Diameter	700	so " rung Nominal	(inches)		17.40	2 :	21.30	24.76	77.57	20.00	20.55	33.85	36.70	30.00
		5	2		2	Diameter Diameter Diameter			(inches)	l	13 02	17.55	17.09	19.80	22.02	36 26	23.52	27.08	29.36	21 07
		Pipe Degian	×		E.	Slope	-	-	3	\mid	0.0050	0000	0.00.0	0.0050	0.0050	0000	0.000	0.000	0.0050	OUCU
			-		R	Length			E		2,700	2700	2,700	2,500	2,500	2 500	200	2,000	2,900	2000
			16	Tourist Inches		Invert	Eloustion		(£		629	610	010	209	236	787	2	2/2	263	מצט
			15	(Inches m		Invert	Floration		E		640	670	25	618	209	200	200	200	5/3	3
		Į.	77	Cumulatha		Wet Weather	Here		(CLS)	1.66	3.74	F 47		9.78	12,71	16.46	20.00	22.00	85'/7	34.34
		Maximum Wet Weather	E	Wet	í	Weather	Flow	{	(CIS)	1.66	2.08	773		3,11	3,13	3.75	2 60	36	2,32	٤
strict	_ suo	Maximum	12	Wat		Weather	Flow		(gpm)	745	934	1223		139/	\$	1685	2514	2307	/907	3125
Utility D	G - 3 EDU Calculatio		11	Wet		Weather	Flow	(49)		1,072,665	1,344,525	1.761.180	440	2,012,333	2,021,220	2,426,055	3 619 875	2 426 665	2,130,003	4.500.465
een Valley Special Utility District	Drainage Area G - 3 EDU Pipe Flow Design Calculations	rage Dry Weather Flow	91	Cumulative		Dry Weather	Flow	(000)		266,805	601,230	1.039.290	100 002	1,339,023	2,042,565	2,646,000	3.546.375	4 401 100	7,701,100	5.520.585
Green Val	Drai Pipe FK	Average Dry	6	۵		Weather	¥ ĕ	(40)		208'992	334,425	438.060	200 636	200,000	502,740	603,435	900.375	954 BAE	200,000	1,119,405
		uo	8	Total		200				1089	1365	1788	2042	200	7027	2463	3675	3480	60.5	4307
		Population	7	Development	-	Density		(ACTRE) (3 FDRI/ACTR) (FDII)	200	3.0	3.0	3.0	3.0	200	3.0	3.0	3.0	9.0	200	2.0
		Area	9	Total	4	Y CO				363	455	296	481		984	821	1225	1163	200	7707
		Contributing Area	S	Right	1	NGC.	Area	~	+	è	187	1	200	1	3	160	387	563	25.5	25
		Ŝ	*	Le 7	4		n Area	(acres)		£	268	452	ÇÇ.	2	4	199	838	9	¥	
		cation	٣	Down	-	Heans	Collection	Polit			2 0 0	- 6 6-3	5	2		9-56	G-7-99	8-50	0	5
		Sewer Main Location	7	ŝ	Change	Suedin	Collection	Point		-1	- 1	-7 -2 -2 -2 -3 -3 -3 -3 -3 -3 -3 -3 -3 -3 -3 -3 -3	ره وردع			96.5	96	C-05-7	ورو	3
		Sew	-	<u>P</u>	٤	3	_		3	200	Pipe GG	Pipe GG	Pine GG				Pipe GG	Ploe GG	00	2

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245 GPD/EDU 735 GPD/EDU 985 GPD/EDU	
Average Dry Weather Flow = Maximum Dry Weather Flow = Maximum Wet Weather Flow =	
GPD capita/EDU EDU/acre GPD/capita	
3.5 3.5 3.5 3.5	3
Residential Single Family Units (EDU) = Population per EDU = Development Average Density = Wastewater Demand = Maximum Flow Peak Factor = Inflow/Infiltration =	Tillings (Tilling good)

Manning's Roughness Coefficient = Percent of Pipe Flowing Full =

Green Valley Special Utility District Summary Costs Proposed Main Wastewater Collection System

Engineer's Opinion of Probable Costs

		=1.5oo P.				
Basin	Tota	l Costs 1 (EDU/acre)	Tota	I Costs 3 (EDU/acre)		Variance
A	4	11,212,950.00	\$	13,229,734.00	\$	2,016,784.00
B	-	3,379,449.00	\$	3,848,841.00	\$	469,392.00
	-	4,151,280.00	\$	4,773,440.00	\$	622,160.00
C	* -	3,072,068.00	4	4,188,876.00	\$	1,116,808.00
D	<u>\$</u>		4	43,682,177.00	4	9,080,364.00
E	<u>\$</u>	34,601,813.00	\$		4	1,509,816.00
F	\$	5,230,109.00	\$	6,739,925.00	\$	710,248.00
G	\$	3,963,086.00	\$	4,673,334.00	\$	
Total	\$	65,610,755.00	\$	81,136,327.00	\$	15,525,572.00

Green Valley Special Utility District Summary Drainage Basin A Engineer's Opinion of Probable Costs

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	Engineer's Opinion of Pro		· · · · · · · · · · · · · · · · · · ·		
Item	Description	Total	Costs 1 (EDU/acre)	Total (Costs 3 (EDU/acre)
	1211 CDD 25 22/0 (01 6)	<u></u>			
1	12" SDR 35, PVC (0'-6' cut)	\$	336,000.00	\$	-
2	15" SDR 35, PVC (0'-6' cut)	\$	533,000.00		169,000.00
3	18" SDR 35, PVC (0'-6' cut)	\$	1,386,000.00	\$	784,000.00
4	21" SDR 35, PVC (0'-6' cut)	\$	720,000.00	\$	1,104,000.00
5	24" SDR 35, PVC (0'-6' cut)	\$	315,000.00	\$	855,000.00
6	27" SDR 35, PVC (0'-6' cut)	\$	250,000.00	\$	550,000.00
7	30" SDR 35, PVC (0'-6' cut)	\$	275,000.00	\$	385,000.00
8	33" SDR 35, PVC (0'-6' cut)	\$	1,425,000.00	\$	-
9	36" SDR 35, PVC (0'-6' cut)	\$	<u> </u>	\$	750,000.00
10	42", PVC (0'-6' cut)	\$	-	\$	2,280,000.00
11	48", PVC (0'-6' cut)	\$	•	\$	-
12	54", PVC (0'-6' cut)	\$	-	\$	-
13	60", PVC (0'-6' cut)	\$		\$	-
14	66", PVC (0'-6' cut)	\$	-	\$	-
15	72", PVC (0'-6' cut)	\$	-	\$	-
		<u> </u>		<u> </u>	
16	48" dia. M.H. W.T. & Bolted (0'-6' cut)	\$	625,000.00	\$	625,000.00
17	Bore and Case Roadways	\$	406,250.00	\$	406,250.00
18	Bore and Case Creek Crossings	\$	250,000.00	\$	250,000.00
19	Trench Safety	\$	125,000.00	\$	125,000.00
20	Sewer Main Television Inspection	\$	1,625,000.00		1,625,000.00
21	Erosion Control Devices	\$	62,500.00	\$	62,500.00
22	Sewer Junction Structure	\$	35,000.00	\$	35,000.00
23	Lift Station	\$	200,000.00	\$	200,000.00
		<u> </u>		<u> </u>	_00,000.00
	Total Construction	\$	8,568,750.00	\$	10,205,750.00
	Contingencies	\$	856,875.00	\$	1,020,575.00
	Total	\$	9,425,625.00	\$	11,226,325.00
		1	,,	· ·	
	Easements	\$	312,500.00	\$	312,500.00
	Easements and Surveys and Acquisition Costs	\$	125,000.00	\$	125,000.00
	Environmental Investigation	\$	93,750.00	\$	93,750.00
	Total Easement Costs	\$	531,250.00	\$	531,250.00
		1		-	
	Basic Engineering	\$	942,562.50	\$	1,122,632.50
:	Survey	\$	125,000.00	\$	125,000.00
(Construction Phase Services	\$	188,512.50	\$	224,526.50
	Total Engineering Costs	\$	1,256,075.00	\$	1,472,159.00
				•	, - ,,
	Total Project Costs	\$	11,212,950.00	\$	13,229,734.00

1	Green Valley Special Utili Drainage Basin A - 1 EE	tility Distri EDU/acre	Utility District 1 EDU/acre				4		
	Engineer's Opinion or Probable Costs	Init o	Unit Quantity	Unit	Unit Price	۴	Total Costs	Item	Ε
E		1							H
-	12" SDR 35. PVC (0'-6' cut)	5	2,600	\$	90.00	₩,	336,000.00	-	4
1	15" SDR 35, PVC (0'-6' cut)	<u>"</u>	8,200	₩.	65.00	60	533,000.00	7	+
1 ~	18" SDR 35, PVC (0'-6' cut)	4	19,800	₩.	20.00	₩.	1,386,000.00	ν,	+
4	21" SDR 35, PVC (0'-6' cut)	Ŧ	000'6	₩.	80.00	45	720,000.00	4 r	+
- 6	24" SDR 35, PVC (0'-6' cut)	<u>"</u>	3,500	₩.	90.00	₩.	315,000.00	v,	+
9	27" SDR 35, PVC (0'-6' cut)	Ŧ	2,500	₩.	100.00	₩.	250,000.00	۱٥	+
-	30" SDR 35, PVC (0'-6' cut)	<u>1</u>	2,500	₩.	110.00	₩.	275,000.00)	+
œ	33" SDR 35, PVC (0'-6' cut)	느	11,400	\$	125.00	S	1,425,000.00	Σ	+
6	36" SDR 35, PVC (0'-6' cut)	<u>"</u>	0	S	150.00	\$	•	۳ څ	
2	42" SDR 35, PVC (0'-6' cut)	<u>"</u>	0	s .	200.00	6	.	3 =	+
Ξ	48" SDR 35, PVC (0'-6' cut)	۳.	0	S	250.00	<i>a</i>	•	1 5	-
12	54" SDR 35, PVC (0'-6' cut)	<u></u>	0	6	300.00	٠,		1 2	u ~
13	60" SDR 35, PVC (0'-6' cut)	5	0	vs.	350.00	60	•	-1 -	
4	66" SDR 35, PVC (0'-6' cut)	5	0	₩.	400.00	·		† L	-
12	72" SDR 35, PVC (0'-6' cut)	当	0	₩.	450.00	8	•	2	_
	Total Length	۳	62,500					ļ.	+
			10,	- 1	000	4	00 000 303	-	<u>_</u>
16	48" dia. M.H. W.T. & Bolted (0'-6' cut)	<u>ا</u>	125	-	00.000,0	A 4	406 250 00	7	
=	Bore and Case Roadways	5	3125	- 1	20.00	4	100,630,00	1 0	
188	Bore and Case Creek Crossings	5	2,500	1	100.00	υn (1	250,000.00	9 9	0 0
19	Trench Safety	5	62,500	- 1	00'7	•	125,000.00	1	n c
20	Sewer Main Television Inspection	5	62,500	.	26.00	65	1,625,000.00	3 7) -
77	Erosion Control Devices	5	62,500	- 1	1,00	5	02,000.00	17	1,
72	Sewer Junction Structure	Ø		ک	35,000.00	· 60	35,000.00	77	7 0
12	Lift Station	Æ	-	\$200	\$ 200,000.00	8	200,000.00	7	2
							0 E60 7E0 00		1
	Total Construction	1				n .	00'06'/2000'	<u>l</u>	2
	Contingencies	10%				1.	00.0,0,000 00.00,000 00.00,000	_	ř
	Total					^	,453,653,00	L	-
		u	62 500	v	00.5	4	312,500.00	L	E
	Fasements	<u>"</u>	62,500	•	2.00	₩	125,000.00		ß
	Easements and surveys and Acquisition costs	<u></u>	62,500		1.50	₩,	93,750.00		ㅁ
	Total Escament Costs	j -		-		4	531,250.00		ř
	local Edsement was							_	+
	Basic Engineering	10%				49	942,562.50		8
	Survey	5	62,500	₩	2.00	S	125,000.00	1	7 (
	Construction Phase Services	7%				1	188,512.50	_	3 ₽
	Total Engineering Costs					s	1,256,075.00	_	+
		1				1	411 212 DED OD	ļ.	1
	Total Project Costs					A .	1,414,930,00	L	+
					di di	1 100	Engineering's heet	LĒ	his cost e

This cost estimate is based on River City Engineering's experience and qualifications, and represents River City Engineering's best judgment. This cost estimate was prepared for feasibility analysis purposes only. River City Engineering does not guarantee that the actual construction cost will not vary from this estimate. Unit prices were used from SAWS average unit price list revised October 2005. Units prices will not remain constraint and will vary due to market variations such as inflation.

	<u> </u>	Utility District 3 EDU/acre	strict cre				4	
	nion of	bable	Probable Costs		Spirot 4	F	Total Coete	Т
Item	Description	Unic	Unit Quantity	5	UNIT PIECE	=	Sign Costs	T
-	12" CDD 35 DVC (0"-6" CIT)	5	0	₩.	90.09	₩.	•	1 1
1	15" SDR 35, PVC (0'-6' cut)	4	2,600	₩.	65.00	\$	169,000.00	0
7 ~	SDR 35, PVC	۳	11,200	₩.	20.00	•	784,000.00	0
7 4	21" SDR 35, PVC (0'-6' cut)	5	13,800	₩	80.00	₩.	1,104,000.00	0
- 6	SDR 35	<u>"</u>	9,500	₩	90.00	₩.	855,000.00	0
י ר	SDR 35	4	5,500	₩.	100.00	₩	550,000.00	0
2	SDR 35, PVC (0'-6'	5	3,500	•	110.00	₩.	385,000.00	0
α	33" SDR 35, PVC (0"-6" cut)	5	0		125.00	₩.	•	7
0	36" SDR 35 PVC (0'-6' Cut)	5	5,000	₩	150.00	₩.	750,000.00	0
1	42" SDR 35, PVC (0'-6' cut)	5	11,400	₩	200.00	₩.	2,280,000,00	0
2	48" SDR 35, PVC (0'-6' cut)	<u>L</u>	0	₩	250.00	₩.	•	
12	54" SDR 35, PVC (0'-6' cut)	5	0	€9	300.00	₩.	1	
1 5	60" SDR 35, PVC (0'-6' cut)	5	0	\$	350.00	•	•	Ţ
4	66" SDR 35. PVC (0'-6' cut)	5	0	₩.	400.00	₩.	•	1
<u>,</u>	72" SDR 35, PVC (0'-6' cut)	5	0	₩	450.00	₩.	•	
3	Total Length	5	62,500					
				- 1				İ
16	48" dia. M.H. W.T. & Bolted (0'-6' cut)	Ę	125		5,000.00	s.	625,000.00	2 9
17	Bore and Case Roadways	5	3125	- 1	130.00	ъ.	405,250.00	2 5
18	Bore and Case Creek Crossings	5	2,500	- 1	100.00	S	250,000.00	2 9
19	Trench Safety	5	62,500	- 1	7.00	ω.	125,000.00	2 5
20	Sewer Main Television Inspection	"	62,500	- 1	26.00	S	1,625,000.00	2 2
21	Erosion Control Devices	5	62,500	vo	1.00	8	62,500.00	2 9
22	Sewer Junction Structure	Æ		₩.	35,000.00	·	35,000.00	2 2
23	Lift Station	ā		\$ 7	\$ 200,000.00	φ.	200,000,002	2
						\$10	\$ 10.205,750.00	Q
	Total Construction	1007					1.020.575.00	2
	Contingencies	0/OT				, -	¢11.226.325.00	Q
	Total			_				
	Facements	4	62,500	₩.	5.00	₩.	312,500.00	8
	Escaments and Surveys and Armisition Costs	4	62,500	₩,	2,00	H	125,000.00	8
	Environmental Investigation	4	62,500	45	1.50		93,750.00	8
	Total Facement Costs					•	531,250.00	9
	Basic Engineering	10%		ш		45	1,122,632.50	2
	Survey	5	62,500	₩.	7.00	\dashv	125,000.	8
	Construction Phase Services	7%				- 1	224,526.50	2
	Total Engineering Costs]		s	1,472,159.00	2
				1		4	613 220 734 00	2
	Total Project Costs			1		•	o'carolo	2
				1				Γ

Green Valley Special Utility District Summary Drainage Basin B Engineer's Opinion of Probable Costs

B

	Engineer's Opinion of Pro				
Item	Description	Total	Costs 1 (EDU/acre)	Total C	Costs 3 (EDU/acre)
<u> </u>	121 CDD 25 51/2 (2)				
1	12" SDR 35, PVC (0'-6' cut)	\$		\$	-
2	15" SDR 35, PVC (0'-6' cut)	\$	169,000.00	\$	-
3	18" SDR 35, PVC (0'-6' cut)	\$	392,000.00		182,000.00
4	21" SDR 35, PVC (0'-6' cut)	\$	560,000.00	\$	224,000.00
5	24" SDR 35, PVC (0'-6' cut)	\$	450,000.00	\$	252,000.00
6	27" SDR 35, PVC (0'-6' cut)	\$		\$	260,000.00
7	30" SDR 35, PVC (0'-6' cut)	\$	-	\$	1,034,000.00
8	33" SDR 35, PVC (0'-6' cut)	\$	-	\$, : ,-=====
9	36" SDR 35, PVC (0'-6' cut)	\$		\$	-
10	42", PVC (0'-6' cut)	\$	-	\$	-
11	48", PVC (0'-6' cut)	\$	_	\$	-
12	54", PVC (0'-6' cut)	\$	_	\$	-
13	60", PVC (0'-6' cut)	\$	-	\$	-
14	66", PVC (0'-6' cut)	\$		\$	-
15	72", PVC (0'-6' cut)	\$	-	\$	-
				<u> </u>	
	4011				
16	48" dia. M.H. W.T. & Bolted (0'-6' cut)	\$	202,000.00	\$	202,000.00
17	Bore and Case Roadways	\$	131,300.00	\$	131,300.00
18	Bore and Case Creek Crossings	\$	80,800.00	\$	80,800.00
19	Trench Safety	\$	40,400.00	\$	40,400.00
20	Sewer Main Television Inspection	\$	525,200.00	\$	525,200.00
21	Erosion Control Devices	\$	20,200.00	\$	20,200.00
22	Sewer Junction Structure	\$	-	\$	
23	Lift Station	\$		\$	-
I					
	Total Construction	\$	2,570,900.00	\$	2,951,900.00
	Contingencies	\$	257,090.00	\$	295,190.00
	Total	\$	2,827,990.00	\$	3,247,090.00
	Easements	\$	101,000.00	\$	101,000.00
	Easements and Surveys and Acquisition Costs	\$	40,400.00	\$	40,400.00
	Environmental Investigation	\$	30,300.00	\$	30,300.00
	Total Easement Costs	\$	171,700.00	\$	171,700.00
	Basic Engineering	\$	282,799.00	\$	324,709.00
	Survey	\$	40,400.00	\$	324,709.00 40,400.00
	Construction Phase Services	\$	56,559.80	\$ \$	40,400.00 64,941.80
	Total Engineering Costs	\$	379,758.80	\$ \$	430,050.80
-		+	31 3 ₁ 7 30.00	4	+30,050.80
	Total Project Costs	\$	3,379,448.80	\$	3,848,840.80